

U.S. DEPARTMENT OF THE INTERIOR
U.S. GEOLOGICAL SURVEY

**Seismic velocities and geologic logs from borehole measurements
at seven strong-motion stations that recorded the
1989 Loma Prieta, California, earthquake**

by

James F. Gibbs, Thomas E. Fumal, and Thomas J. Powers ¹

Open-File Report 94-222

This report is preliminary and has not been reviewed for conformity with U.S. Geological Survey editorial standards or with the North American Stratigraphic Code. Any use of trade, product, or firm names is for descriptive purposes only and does not imply endorsement by the U.S. Government.

¹ U.S. Geological Survey, MS 977, Menlo Park, CA 94025

TABLE OF CONTENTS

	Page
Introduction	1
Field Measurements	3
Geologic Logs	3
Travel-time Data	4
Data Interpretation and Processing	5
Summary of Results	7
S-wave Velocities	7
P-wave Velocities	10
Acknowledgments	13
References	14
Appendices—Detailed Results:	
Beach Park Boulevard (Foster City)	16
Fremont	31
Hayward City Hall	43
Larkspur Ferry	54
Pacific Park Plaza (Emeryville)	66
Stanford Linear Accelerator (SLAC)	79
Sunnyvale Colton Avenue	92

Seismic velocities and geologic logs from borehole measurements
at seven strong-motion stations that recorded the
1989 Loma Prieta, California, earthquake

by

Gibbs, James F., Thomas E. Fumal, and Thomas J. Powers

INTRODUCTION

The Loma Prieta earthquake of October 17, 1989 (1704 PST) was recorded at 131 strong-motion stations located through-out the San Francisco Bay area (Maley et al., 1989, Shakal, et al., 1989). This data set has enormous value for engineering and seismological studies regarding earthquake ground motions. Using damage to man-made structures from the 1906 San Francisco earthquake, Lawson (1908) recognized that ground-motion intensity could be correlated with differences in local site geology. In order to quantify the effect of local geology (e.g., Borcherdt, 1970; Borcherdt and Gibbs, 1976) on ground motions from the 1989 earthquake, detailed geologic and geophysical data are needed. To plan the acquisition of these data a meeting was held on July 6, 1990 at the USGS in Menlo Park, California. Eighteen scientists and engineers representing thirteen institutions attended the meeting to coordinate drilling and data aquisition plans at strong-motion stations.

This is the third of a planned series of four reports (Gibbs et al., 1992, 1993) detailing the results of borehole measurements at strong-motion stations recording significant ground motions during the 1989 Loma Prieta earthquake. This report contains the results of the field effort by the USGS for the following seven boreholes located near strong-motion stations operated by the U.S. Geological Survey (Figure 1).

1. Beach Park Boulevard (Foster City)
2. Fremont
3. Hayward City Hall
4. Larkspur Ferry
5. Pacific Park Plaza (Emeryville)
6. Stanford Linear Accelerator (SLAC)
7. Sunnyvale Colton Avenue

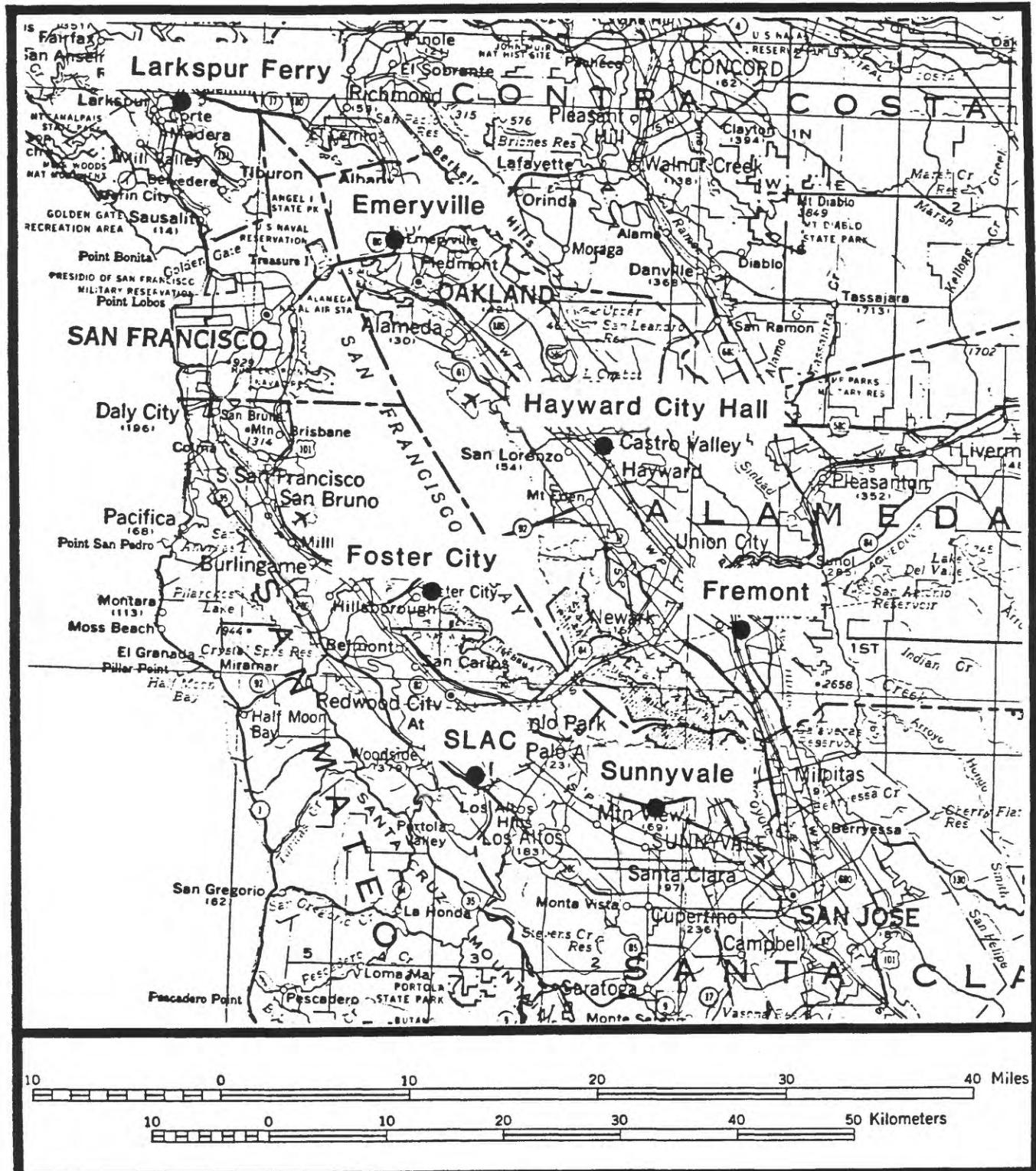


Figure 1. Regional map showing the locations of boreholes (solid circles) included in this report.

FIELD MEASUREMENTS

Drilling and Sampling Procedures

Boreholes were located as close as possible to the accelerograph stations that recorded the strong-motion from the Loma Prieta Earthquake. In the case of Beach Park Boulevard (Foster City) and Stanford Linear Accelerator (SLAC) the closest available drill site was located approximately 200 meters from the strong-motion instrument. Fremont and Sunnyvale Colton Avenue boreholes were drilled approximately 100 meters from the instrument location. Hayward City Hall borehole was located approximately 30 meters from the accelerograph station and Pacific Park Plaza (Emeryville) and Larkspur Ferry boreholes were within 10 meters of the accelerograph station.

At each site a hole approximately 6 to 10 inches in diameter was drilled using rotary-wash drilling with bentonite mud. For purposes of economy, samples were not taken in any of the boreholes.

The borings at all stations in this report were cased with 3-inch inside-diameter, class 200, polyvinyl-chloride pipe capped at the bottom.

The annular space around the casing was tremie grouted by pumping a water-cement-bentonite mixture through a 1-inch steel pipe inserted next to the casing. This provides good coupling between the casing and the wall of the borehole, and provides a sanitary seal preventing contamination of ground water. Grouting was done in stages of about 50-60 meters to prevent collapse of the casing.

Geologic Logs

Geologic logs are based on descriptions of drill cuttings, samples, reaction of the drill rig, and inspection of nearby outcrops. Sediment samples are described using the field techniques of the Soil Conservation Service (1951). Descriptions include sediment texture, color, and the amount and size of coarse fragments. Texture refers to the relative proportions of clay, silt, and sand particles less than 2 millimeters in diameter. This is determined visually and by feel without using laboratory tests. As such, this system is easier to use in the field than other classification systems. The dominant color of the sediment and prominent mottles are determined from the Munsell soil color charts.

Descriptions of rock samples include rock name, weathering condition, color, grain

size, hardness, and fracture spacing. Classifications of rock hardness and fracture spacing are those used by Ellen et al., (1972) in describing hillside materials in San Mateo County, California.

Most information needed for describing relatively well-sorted soils and such properties of rock as lithology, color, and hardness are readily obtained from cuttings. Inspection of samples and nearby outcrops is necessary for determining the nature of poorly-sorted materials and fracture spacing. Reaction of the drill rig is useful in determining approximate sediment texture and in determining degree of fracturing because the rate of penetration in rock is highest for very closely fractured and crushed materials and drilling roughness generally is at a maximum in closely to moderately fractured rock. In-situ consistency of soil is determined largely from standard penetration measurements and rate of drill penetration.

Travel-time Data

Shear waves* were generated at the ground surface by an air-powered horizontal hammer (Liu, et al., 1988) striking anvils attached to the ends of a 2.3-meter-long aluminum channel. The hammer can be driven in both horizontal directions to generate positive and negative shear pulses. The switch that determines zero time is a piezo-electric sensor attached to the shear source. The source is offset from the borehole to prevent the direct arrival from traveling down the grout next to the casing. The source offset is 2 to 5 meters depending on the depth of the borehole. Shallow holes (30 meters or less) are generally offset 2 meters, while boreholes deeper than approximately 100 meters are offset 5 meters. Travel times are corrected (for slant offset) to vertical by the cosine of the angle of ray incidence.

P-waves are made by striking a steel plate with a sledge hammer at the same intervals described above. The recorder is triggered by the sledge hammer making electrical contact with the steel plate.

Measurements are made by lowering a single three-component geophone into the bore-hole and clamping it to the casing-wall with an electrically-actuated lever arm. A second

* In this report shear-wave(s) and S-wave are used interchangeably as well as compressional-wave and P-wave.

three-component geophone is placed at the surface approximately 30 centimeters from the shear source and is used as a check of the zero time determined by the triggering of the recorder by the contact switch. All recordings for this group of stations were made at 2.5 meter intervals. The data are recorded on magnetic tape cassettes in digital form on a twelve-channel recording system.

DATA INTERPRETATION and PROCESSING

The flow-chart, Figure 2, describes the processing and interpretation procedures. The magnetic tape cassette contains 18 recorded traces from each depth. These include data from the surface three component geophone and the downhole three-component geophone. There are a total of 6 traces for each source type (positive horizontal, negative horizontal, and vertical). As mentioned previously, the surface geophone is used only to check timing.

The orientation of the downhole geophone cannot be controlled when moving from one depth to the next, so that horizontal components are not generally oriented parallel and perpendicular to the source. This causes slight phase shifts, timing differences and amplitude variations. To minimize these effects, when timing shear-wave arrivals, the horizontal components are combined (rotated) to obtain a single component of motion. The direction of motion is determined by maximizing the integral square amplitude within a time interval containing the shear wave (Boatwright et al., 1986). Rotated traces are plotted on a 20-inch computer monitor and the first shear-wave arrival is timed for each of the horizontal rotated traces. Two arrival times are obtained from picks of the first S-wave arrival from oppositely directed horizontal impacts. Timing of the arrivals is done to one millisecond precision. The two time-picks are not always identical, due to interfering waves obscuring the first shear-arrival, slight phase shifts, or amplitude differences. If the time difference is greater than about 5 milliseconds a mistake in phase correlation (perhaps due to a reversed trace, noise etc.) can be suspected and a repick may be necessary. The two picks are averaged for velocity determinations. On clear traces one-millisecond picking accuracy can be maintained; however, because of lower signal-to-noise ratios and interfering waves in the deeper sections of the boreholes, this accuracy cannot always be achieved. In the inversion for shear-velocity the arrivals are weighted by the inverse of an assigned normalized variance. A normalized standard deviation of 1 was assigned to the

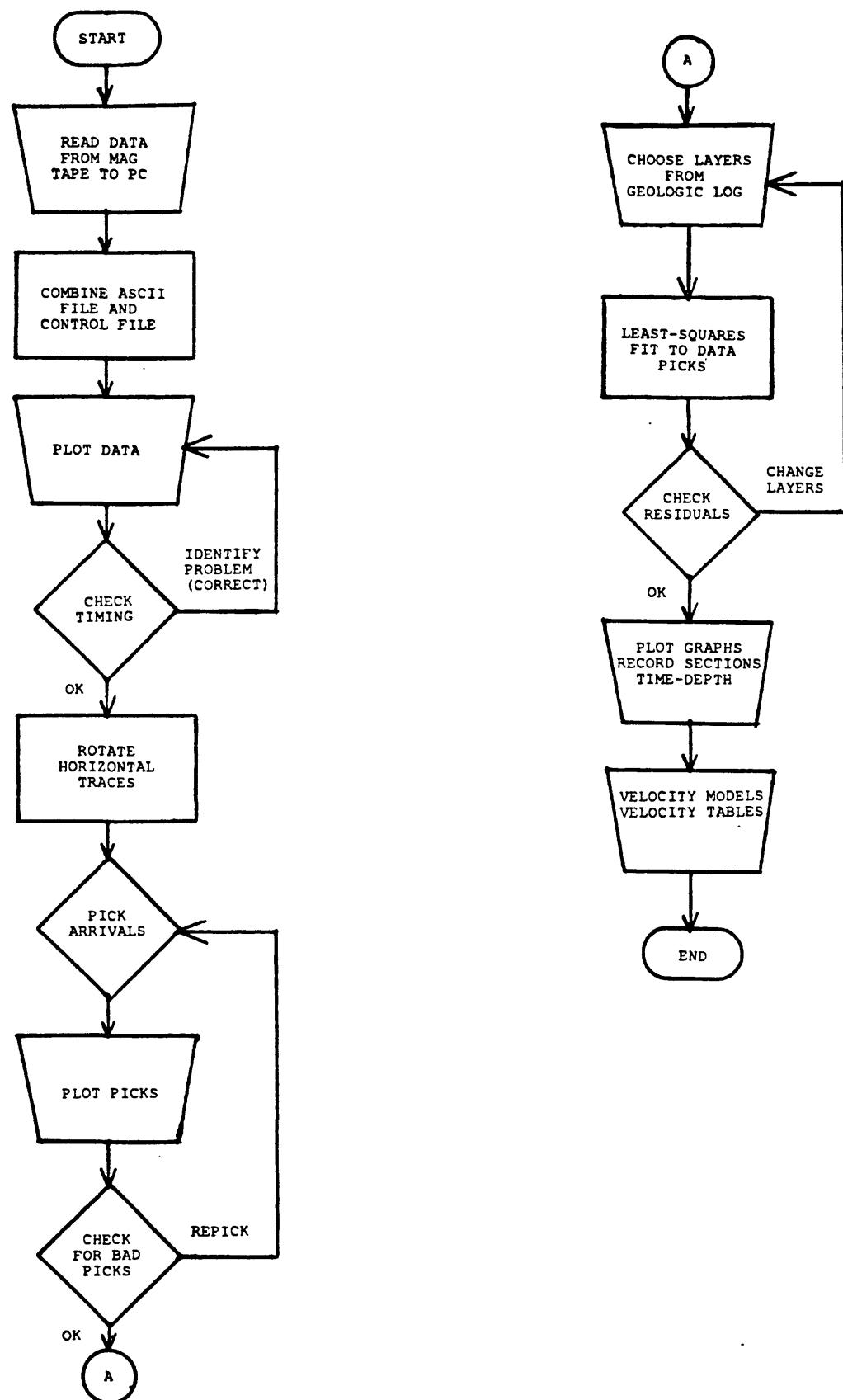


Figure 2. Flow-chart outlining the data processing and interpretation steps.

accurate picks and values ranging up to 5 were assigned to the others.

For determining the final velocity model there are a number of ways to proceed. In our earlier work (e.g., Gibbs et al., 1975) we determined the initial layer boundaries from the travel time plots by eye and then added or subtracted layers based on geologic boundaries consistent with the data. We also required at least three data points in each layer. This requirement limited the velocity determination to layers greater than 7.5 meters in thickness. The problem with this procedure is that a mismatch (overlap or underlap) of the line segments sometimes occurred at the intersections of the layers, resulting in a discontinuous travel time curve. To address this problem we are now using a least-squares program (LFIT, Press et al., 1992) that fits the travel time data with line segments hinged at each selected layer boundary from the surface (forced through zero) to the bottom data point. Initial layer boundaries are chosen from the geologic log and are adjusted, if necessary, to reduce residuals and for consistency with the data. The S-wave travel time data are analyzed first; layer boundaries are initially the same for the P-wave model, and are then adjusted, if necessary, by adding a layer for the water table or reducing the number of layers. The velocity plots (e.g., Figure 13) show upper and lower bounds which approximate 68% confidence limits. These bounds are not symmetrical because they are based on the standard deviation of the slope of the least-squares line fit to the travel time plots (the inverse of the velocity).

SUMMARY OF RESULTS

S-wave velocities

Figure 3 summarizes S-wave velocities at three sites; Beach Park Boulevard (Foster City), Pacific Park Plaza (Emeryville), and Larkspur Ferry. These sites have varied thicknesses of soft bay mud near the surface with S-velocities less than 110 meters/second. Pacific Park Plaza has the thinnest mud section (3 meters), followed by Larkspur Ferry (14 meters) and Beach Park Boulevard (21 meters). Beach Park Boulevard has a low velocity layer at the surface because no artificial fill has been added; Pacific Park Plaza and Larkspur Ferry sites have artificial fill overlaying bay mud resulting in initial velocities of 279 and 353 meters/second, respectively. The S-velocities below the mud are due to varying, of the textures (grain size), the compaction, and the cementation of the sediments, this

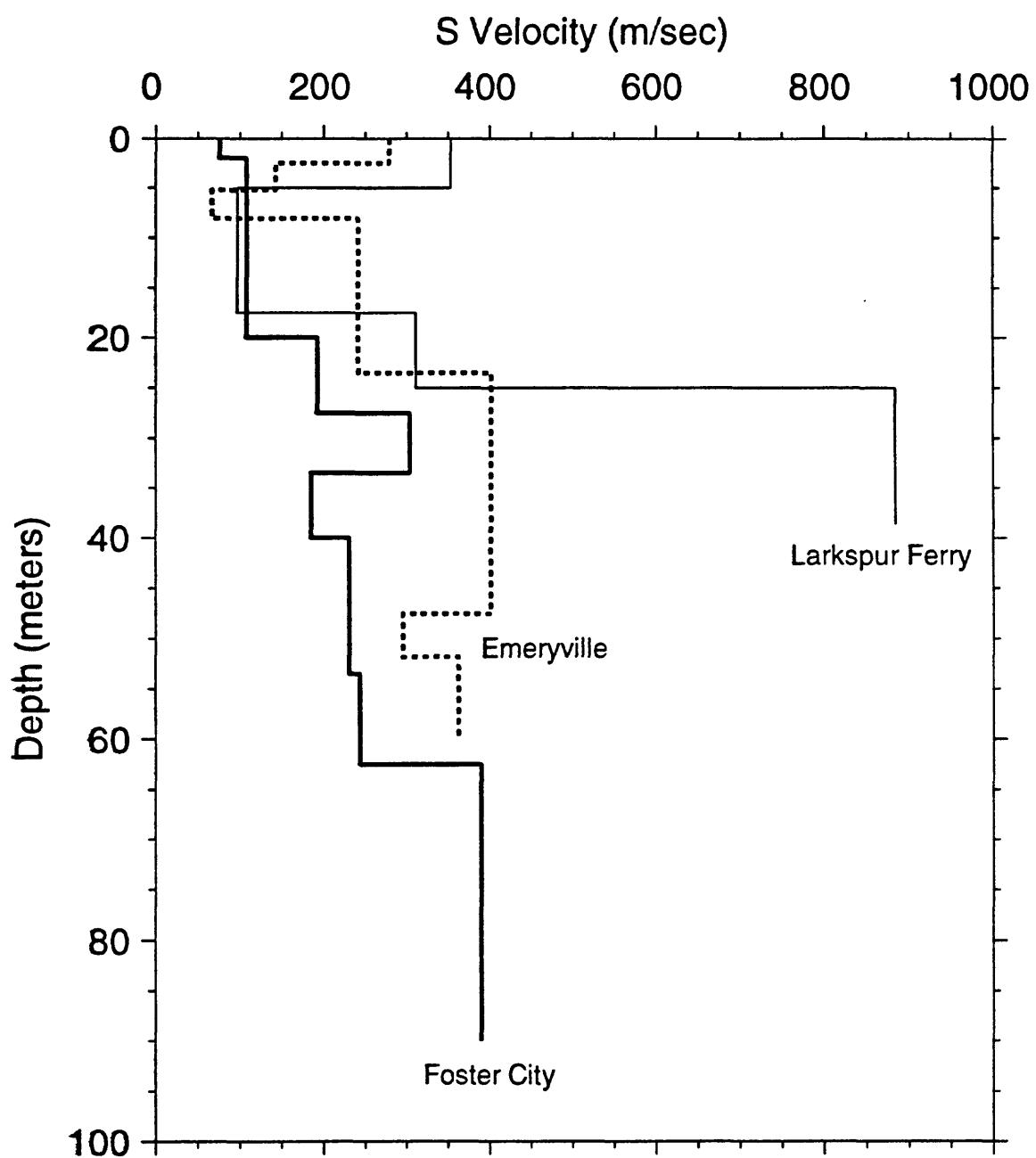


Figure 3. S-wave velocity models superimposed for comparison. The three sites shown have Holocene Bay Mud deposits near the surface and are located close to the margin of San Francisco Bay.

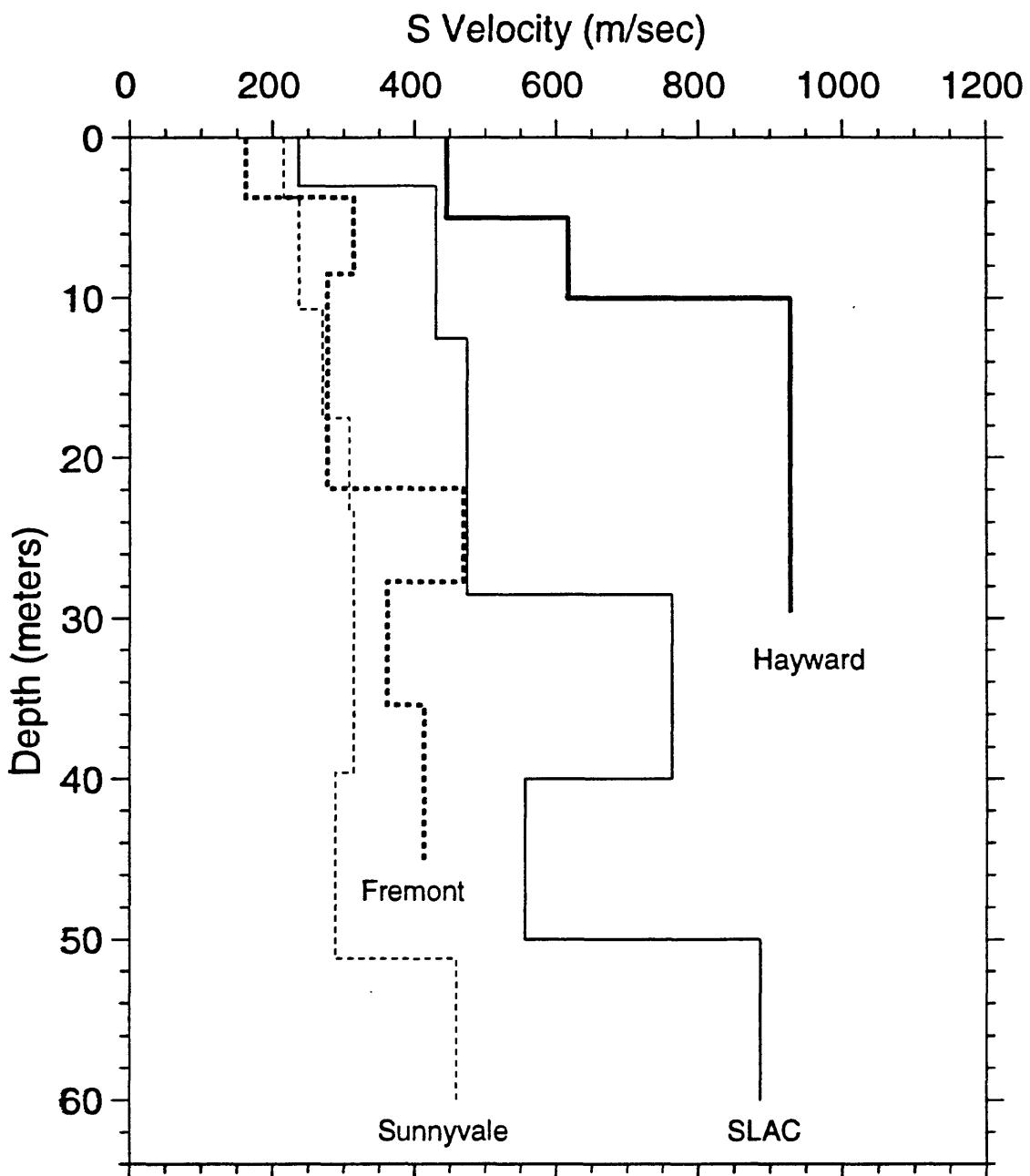


Figure 4. S-wave velocity models superimposed for comparison. In general, these four sites do not have Holocene Bay Mud deposits and as a result have higher S-wave velocities in the upper sections of the borehole.

variation is typical of much of the near-surface geology close to San Francisco Bay.

Figure 4 shows the summaries of S-velocities at four sites; Hayward City Hall, Fremont, SLAC and Sunnyvale Colton Avenue. Fremont and Sunnyvale Colton Avenue, located on the flatlands of the valley, have similar velocities. The S-velocities are higher at SLAC which is farther from San Francisco Bay and in the foothills of the Santa Cruz Mountains. Higher velocities are encountered at the Hayward City Hall site which is underlain by weathered rock (617 meters/second) and by fresh rock (928 meters/second) below 10 meters depth.

P-wave velocities

Figures 5 and 6 summarizes the P-wave velocities at three sites near the margins of San Francisco Bay and at four sites farther inland, respectively. There is a poorer correlation between P-wave velocity and lithology than S-wave velocity and lithology because P-wave velocity is strongly affected by degree of saturation. Note that even though saturated, the P-wave velocities measured in the bay mud are less than the velocity of P-waves in water (≈ 1500 meters/second). The explanation for this may be that the presence of trapped gas (Brandt, 1960) has reduced the P-wave velocity (e.g. air, methane from decaying organic matter).

The appendix lists the detailed results, organized alphabetically by borehole. Figures and tables for each site are arranged in the following order:

1. location map
2. geologic log
3. record sections
4. time-depth graph
5. velocity profiles
6. velocity tables

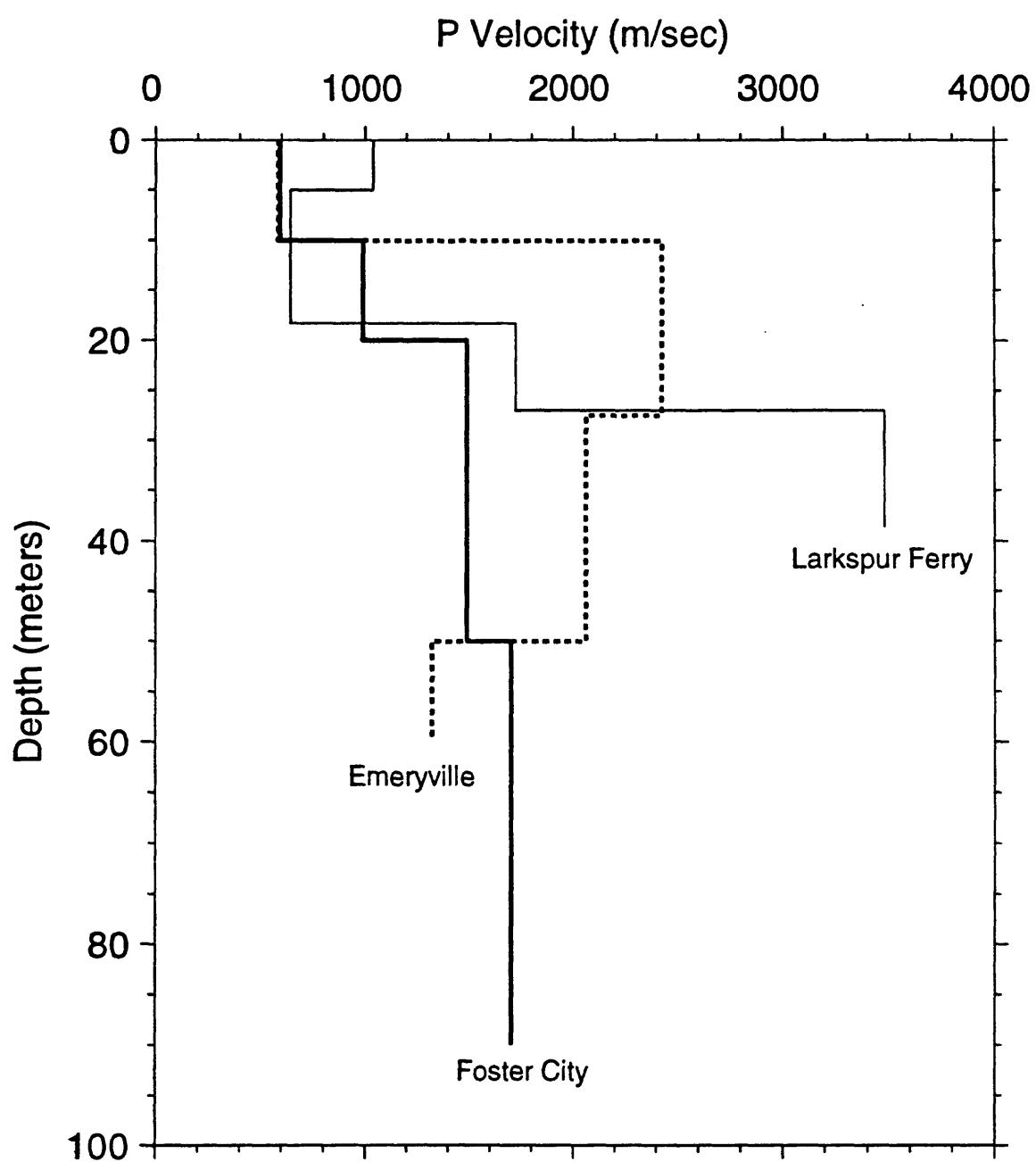


Figure 5. P-wave velocity models superimposed for comparison.

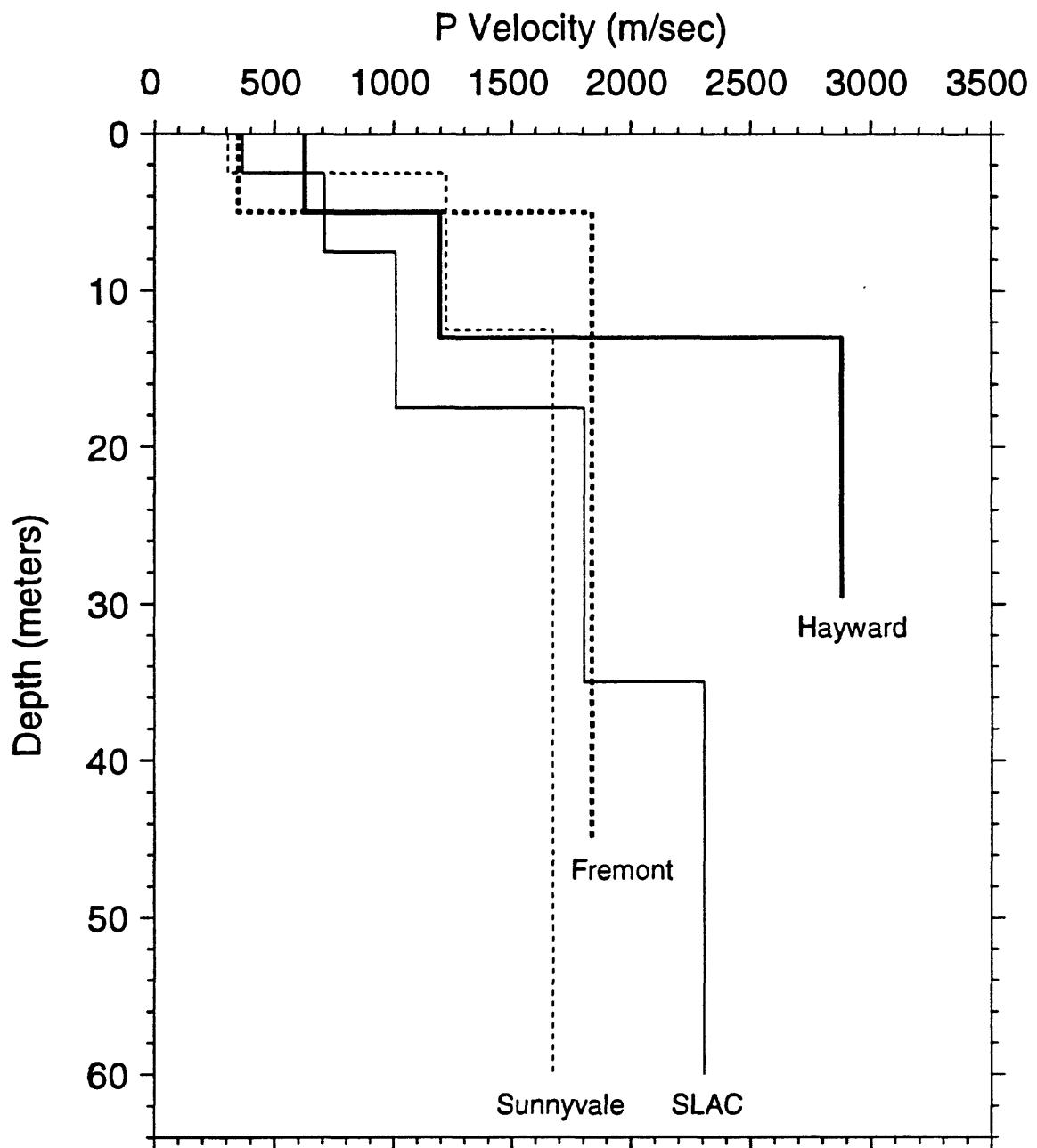


Figure 6. P -wave velocity models superimposed for comparison.

ACKNOWLEDGMENTS

We wish to thank Mr. Robert Westerlund of the *USGS* for building the electrically actuated clamp for the borehole geophone and Dr. Hsi-Ping Liu of the *USGS* for designing the shear-wave generator. In addition, we were assisted in the field by Mr. Michael Carter, and Miss Aina Fox of the *USGS*.

REFERENCES

- Boatwright, John, R. Porcella, T. Fumal, and Hsi-Ping Liu, 1986, Direct estimates of shear wave amplification and attenuation from a borehole near Coalinga, California: *Earthquake Notes*, v. 57, p. 8.
- Borcherdt, R. D., 1970, Effects of local geology on ground motion near San Francisco Bay: *Bull. Seismo. Soc. Am.*, v. 60, p. 29-61.
- Borcherdt, Roger D., and James F. Gibbs, 1976, Effects of local geological conditions in the San Francisco Bay region on ground motions and the intensities of the 1906 earthquake: *Bull. Seismo. Soc. Am.*, v. 66, p. 467-500.
- Brandt, H., 1960, Factors affecting compressional wave velocity in unconsolidated marine sediments: *Acoustical Soc. Am. Jour.*, v. 32, p. 171-179.
- CoPlot, Scientific Graphics Software, CoHort Software, P.O. Box 1149, Berkeley, CA 94701.
- Ellen, S. D., C. M. Wentworth, E. E. Brabb, and E. H. Pampeyan, 1972, Description of geologic units, San Mateo County, California: Accompanying U.S. Geological Survey, Miscellaneous Field Studies Map, MF-328.
- Gibbs, James F., Thomas E. Fumal, David M. Boore, and William B. Joyner, 1992, Seismic velocities and geologic logs from borehole measurements at seven strong-motion stations that recorded the Loma Prieta earthquake: U.S. Geological Survey, Open-File Report 92-287.
- Gibbs, James F., Thomas E. Fumal, and Thomas J. Powers, 1993, Seismic velocities and geologic logs from borehole measurements at eight strong-motion stations that recorded the 1989 Loma Prieta, California, earthquake: U.S. Geological Survey, Open-File Report 93-376.
- Gibbs, James F., Thomas E. Fumal, and Roger D. Borcherdt, 1975, In-situ measurements of seismic velocities at twelve locations in the San Francisco Bay region: U.S. Geological Survey, Open-File Report 75-564, 87p.
- Lawson, A. C., (chairman), 1908, The California earthquake of April 18, 1906: Report of the State Earthquake Commission, Carnegie Inst. Washington.
- Liu, Hsi-Ping, Richard E. Warrick, Robert E. Westerlund, Jon B. Fletcher, and Gary L. Maxwell, 1988, An air-powered impulsive shear-wave source with repeatable signals: *Bull. Seismo. Soc. Am.*, v. 78, p. 355-369.
- Maley, R., A. Acosta, F. Ellis, E. Etheredge, L. Foote, D. Johnson, R. Porcella, M. Salsman, and J. Switzer, 1989, U.S. Geological Survey strong-motion records from the northern California (Loma Prieta) earthquake of October 17, 1989: U.S. Geological Survey, Open-File Report 89-568.
- Powers, Thomas J., and Thomas E. Fumal, 1993, Geologic logs from 25 boreholes near strong motion accelerographs that recorded the 1989 Loma Prieta, California, earthquake: U.S. Geological Survey, Open-File Report 93-502.

Press, William H., Brian P. Flannery, Saul A. Teukolsky, and William T. Vetterling, 1992, Numerical Recipes, the art of scientific computing, General Linear Least Squares: Cambridge University Press, Cambridge, p. 665-670.

Scherbaum, Frank and James Johnson, 1990, Programmable Interactive Toolbox for Seismological Analysis (PITSA): in IASPEI software library, Edited by W. H. K. Lee, distributed by Seismo. Soc. Am., El Cerrito, California 94530.

Shakal, A., M. Huage, M. Reichle, C. Ventura, T. Cao, R. Sherburne, M. Savage, R. Darragh, and G. Peterson, 1989, CSMIP strong-motion records from the Santa Cruz Mountains (Loma Prieta), California earthquake of 17 October 1989: California Department of Conservation, Division of Mines and Geology, Report OSMS 89-06.

Soil Survey Staff, 1951, U.S. Department of Agriculture Handbook 18: U.S. Government Printing Office, Washington D.C. 20402, 503p.

Terzaghi, Karl, and Ralph B. Peck, 1967, Soil mechanics in engineering practice: John Wiley and Sons, New York, 2nd edition.

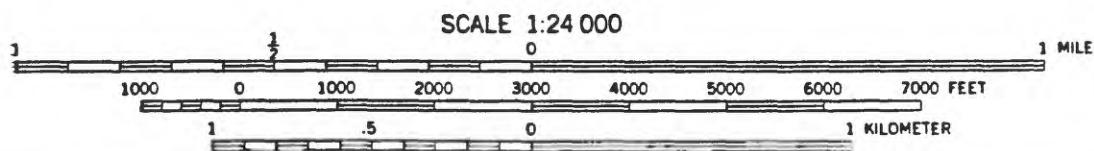
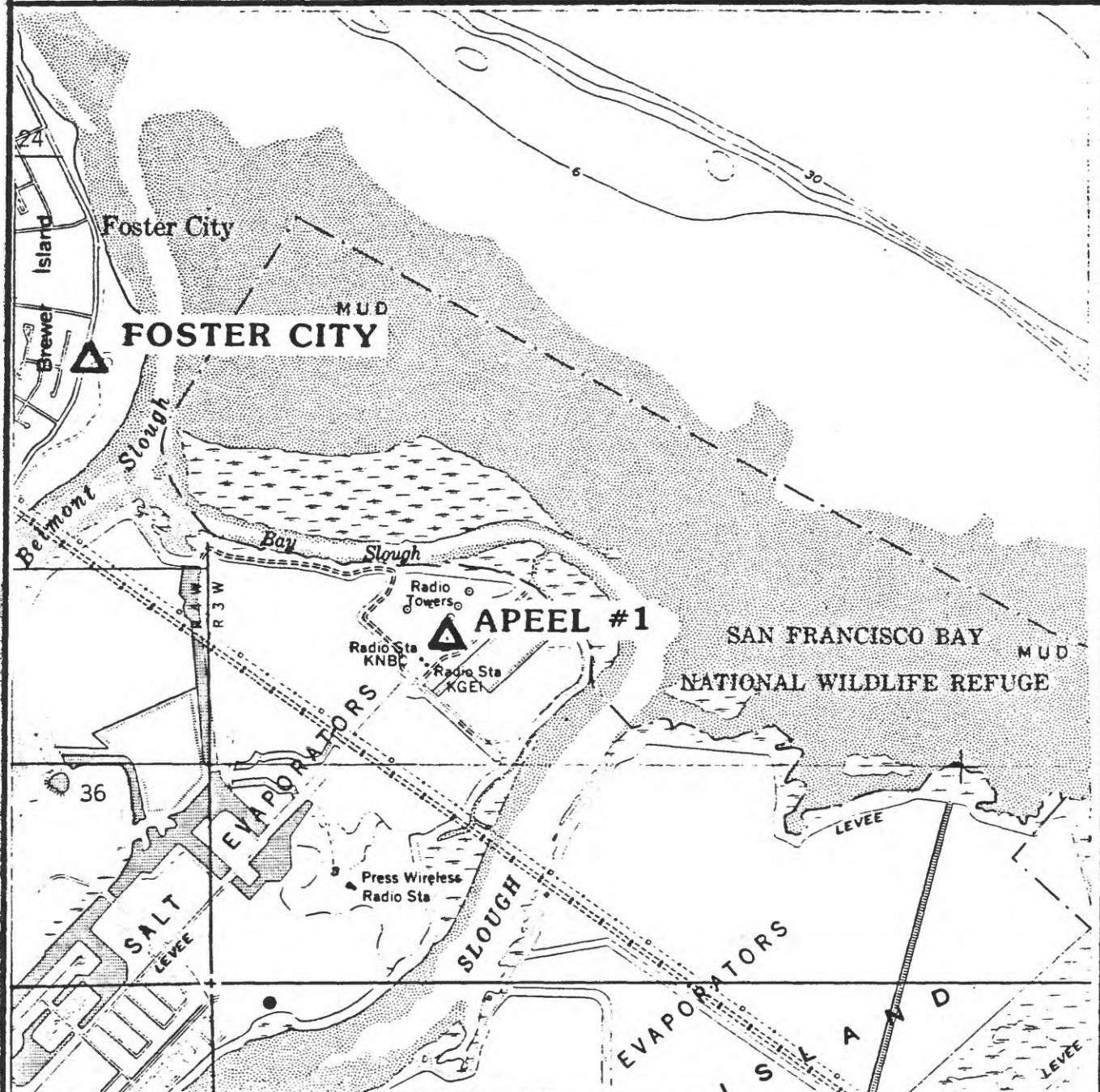


Figure 7. Site location map for the borehole at Beach Park Boulevard (Foster City) and APEEL #1 (not in this report). The borehole at Beach Park Boulevard is located approximately 300 meters from the strong-motion recorder.

Definitions of terms used for descriptions of sedimentary deposits and bedrock materials

Rock hardness: response to hand and geologic hammer: (Ellen et al., 1972)

hard - hammer bounces off with solid sound
 firm - hammer dents with thud, pick point dents or penetrates slightly
 soft - pick points penetrates
 friable material can be crumbled into individual grains by hand.

Fracture spacing: (Ellen et al., 1972)

cm	in	fracture spacing
0-1	0-1/2	v. close
1-5	1/2-2	close
5-30	2-12	moderate
30-100	12-36	wide
> 100	> 36	v. wide

Weathering:

Fresh: no visible signs of weathering

Slight: no visible decomposition of minerals, slight discoloration

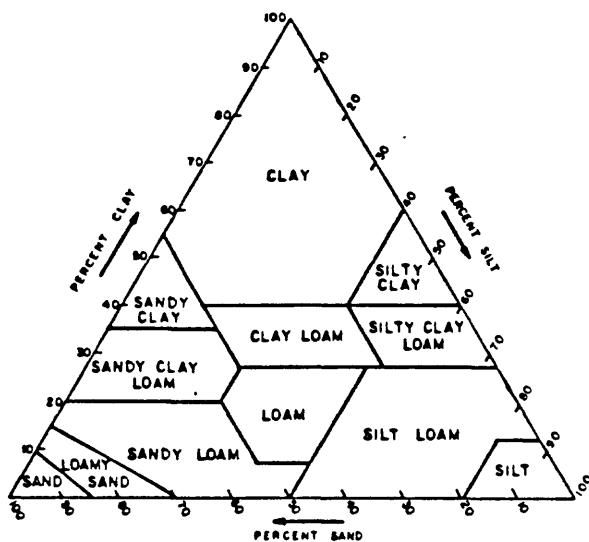
Moderate: slight decomposition of minerals and disintegration of rock, deep and thorough discoloration

Deep: extensive decomposition of minerals and complete disintegration of rock but original structure is preserved.

Relative density of sand and consistency of clay is correlated with penetration resistance: (Terzaghi and Peck, 1948)

blows/ft.	relative density	blows/ft.	consistency
0-4	v. loose	<2	v. soft
4-10	loose	2-4	soft
10-30	medium	4-8	medium
30-50	dense	8-15	stiff
>50	v. dense	15-30	v. stiff
		>30	hard

Texture: the relative proportions of clay, silt, and sand below 2mm. Proportions of larger particles are indicated by modifiers of textural class names. Determination is made in the field mainly by feeling the moist soil (Soil Survey, Staff, 1951).



Color: Standard Munsell color names are given for the dominant color of the moist soil and for prominent mottles.

Types of samples

SP - Standard Penetration 1 + 3/8 in in ID sampler)

S - Thin-wall push sampler

O - Osterberg fixed-piston sampler

P - Pitcher Barrel sampler

CH - California Penetration (2 in ID sampler)

DC - Diamond Core

Figure 8. Explanation of geologic logs.

SITE: BEACH PARK BLVD. (FOSTER CITY)

DATE: 1/22/91

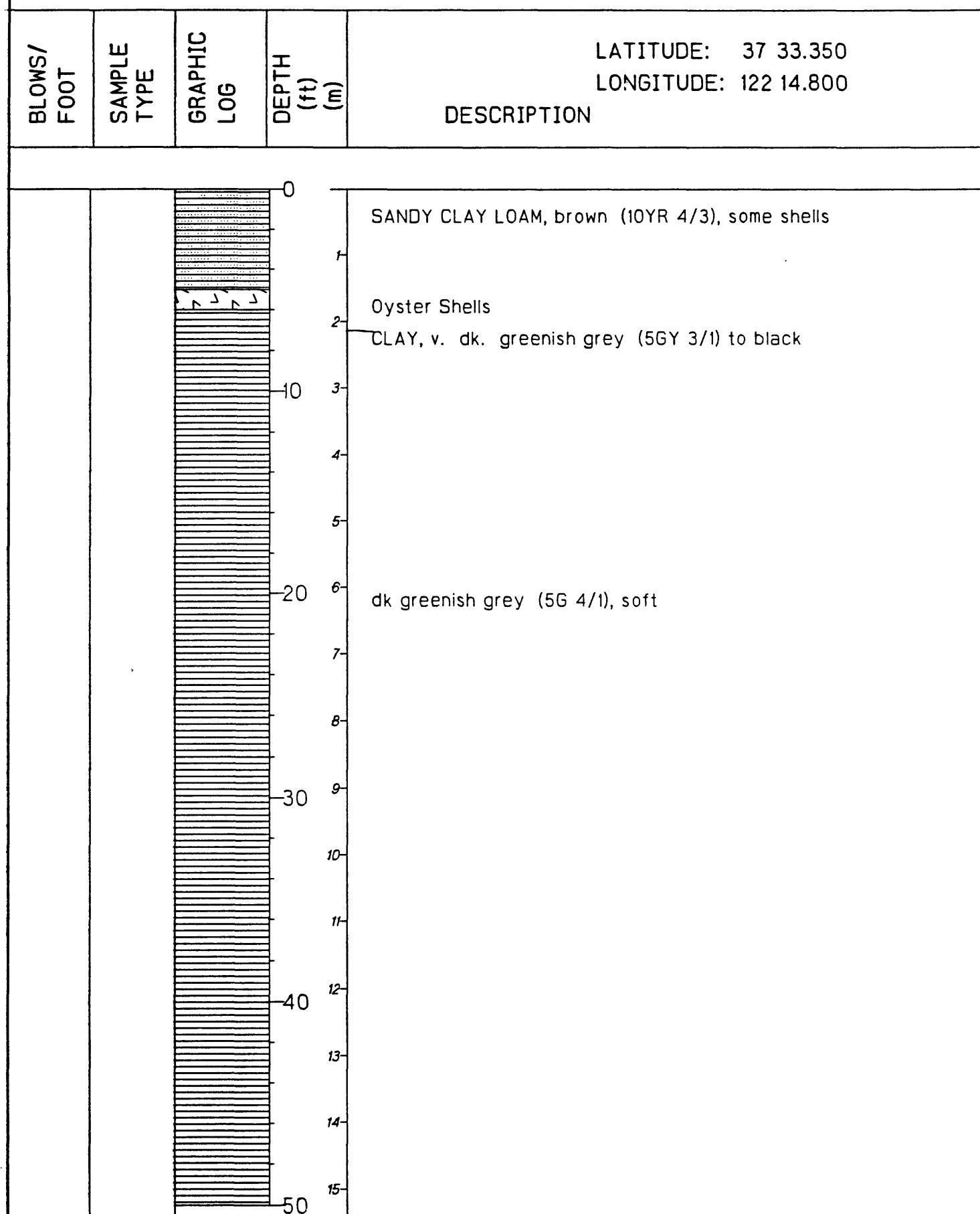


Figure 9. Geologic log of Beach Park Boulevard (Foster City) borehole.

SITE: BEACH PARK BLVD. (FOSTER CITY)

DATE: 1/22/91

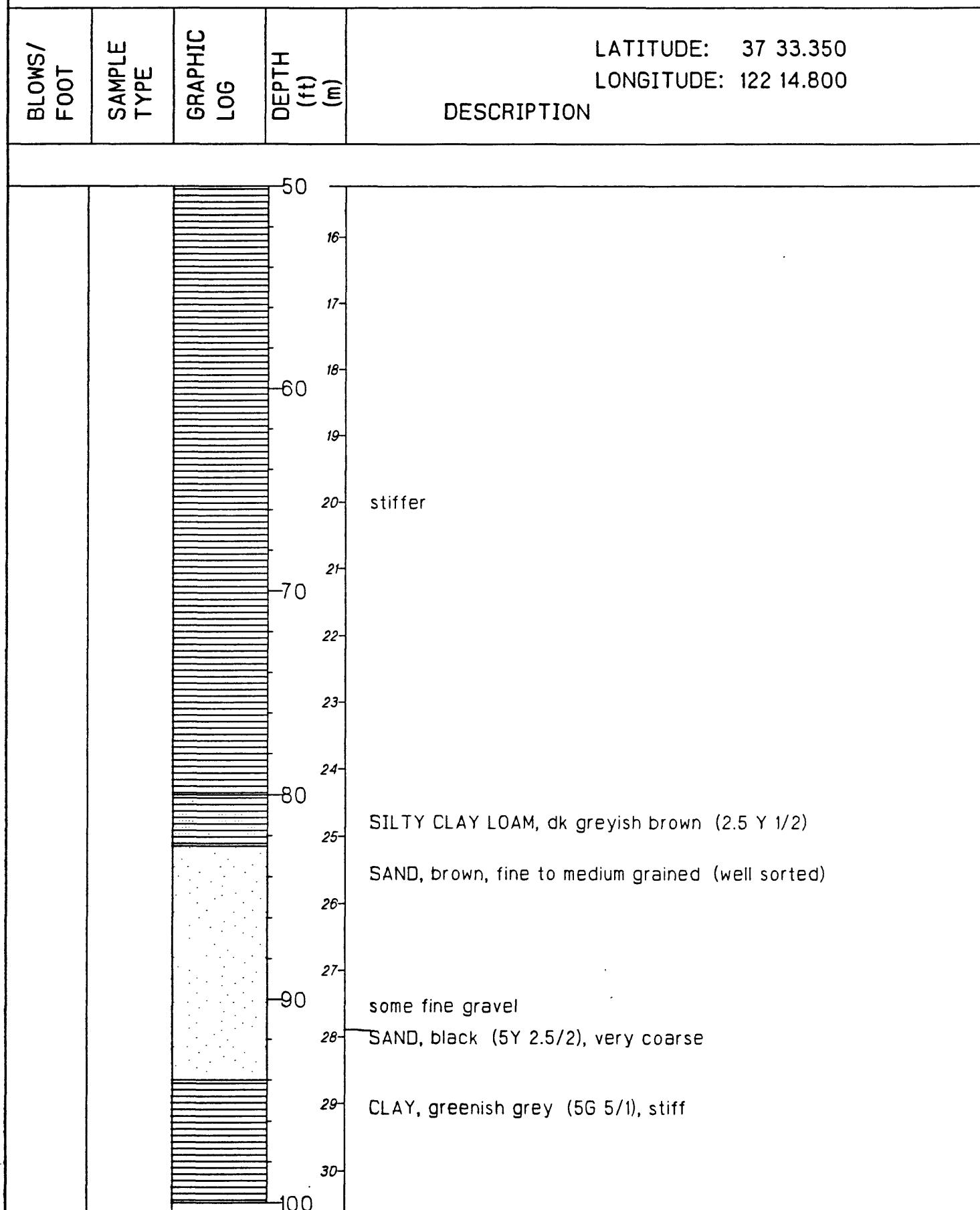


Figure 9. (Continued).

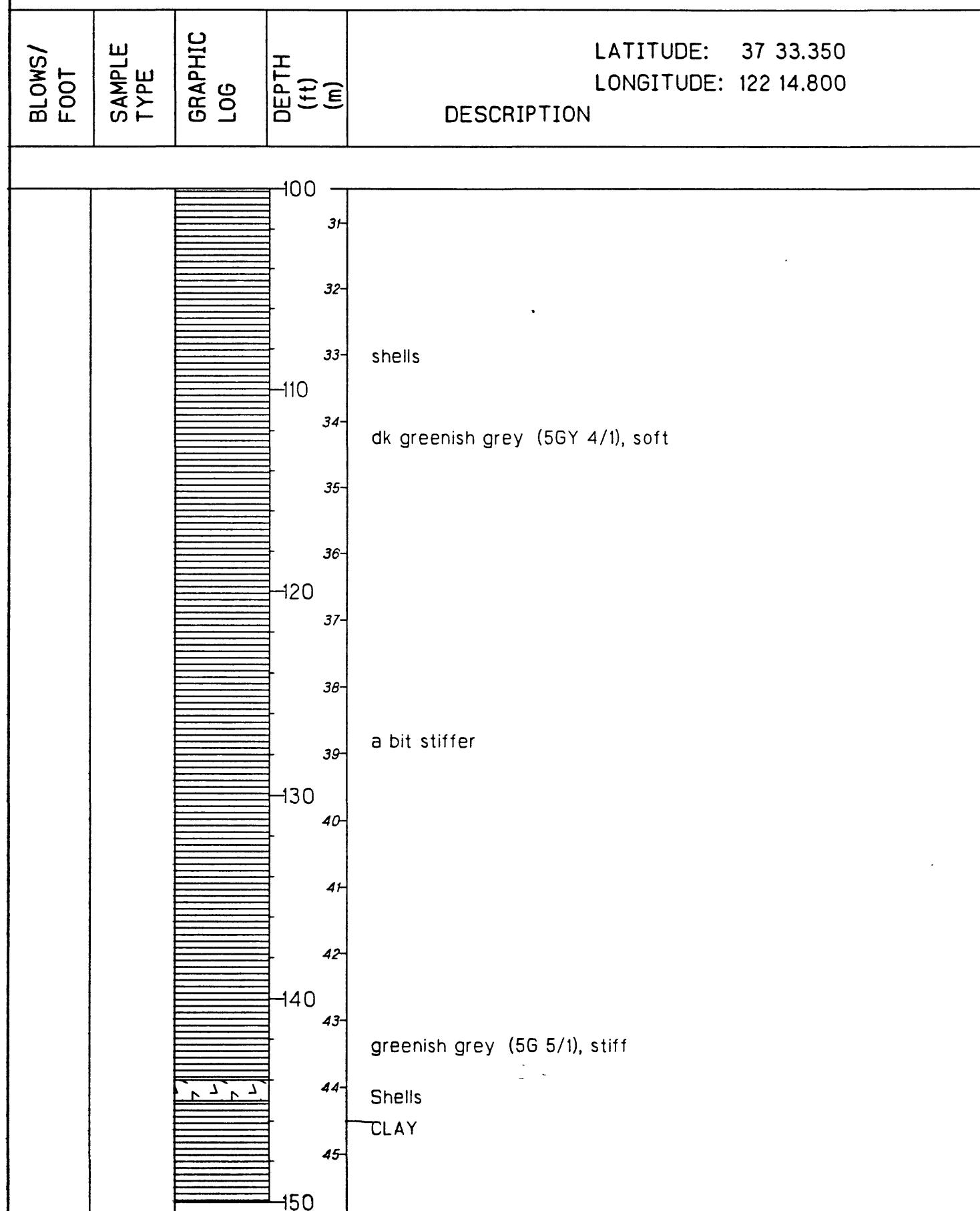


Figure 9. (Continued).

SITE: BEACH PARK BLVD. (FOSTER CITY)

DATE: 1/22/91

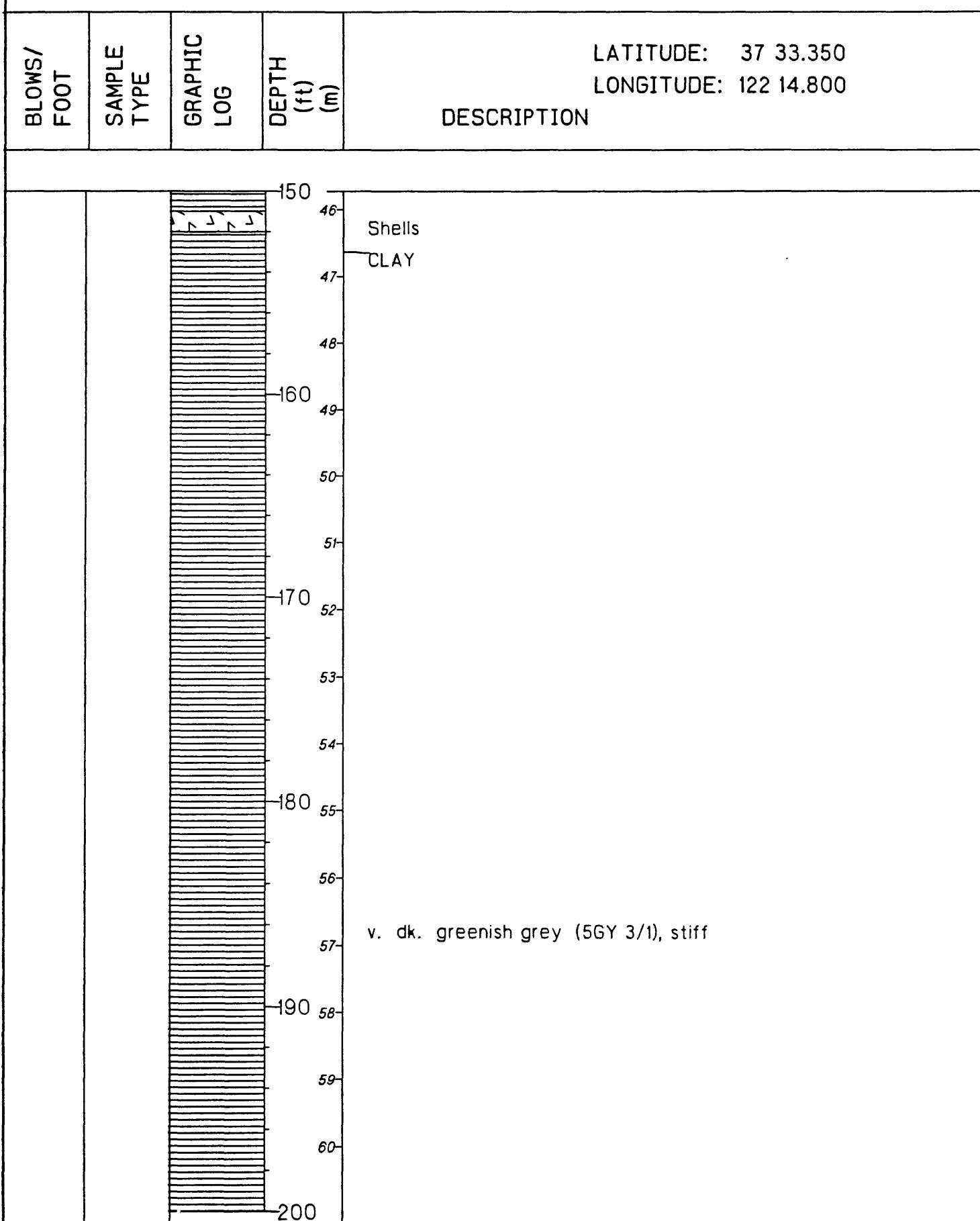


Figure 9. (Continued).

SITE: BEACH PARK BLVD. (FOSTER CITY)

DATE: 1/22/91

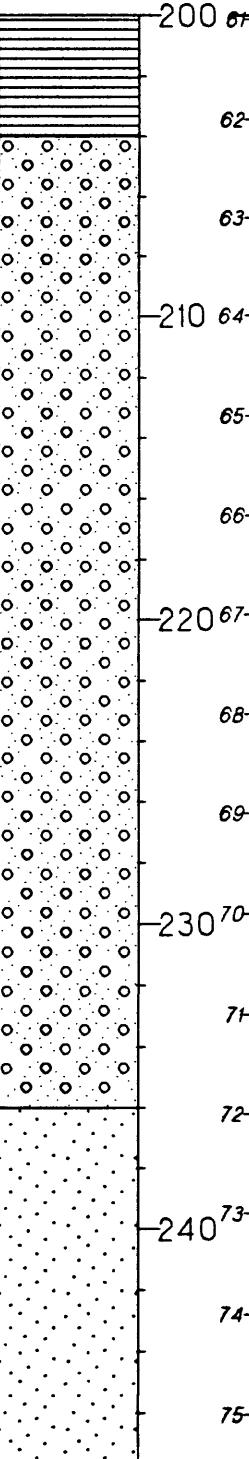
BLOWS/ FOOT	SAMPLE TYPE	GRAPHIC LOG	DEPTH (ft) (m)	DESCRIPTION
				LATITUDE: 37 33.350 LONGITUDE: 122 14.800
				DESCRIPTION
				

Figure 9. (Continued).

SITE: BEACH PARK BLVD. (FOSTER CITY)

DATE: 1/22/91

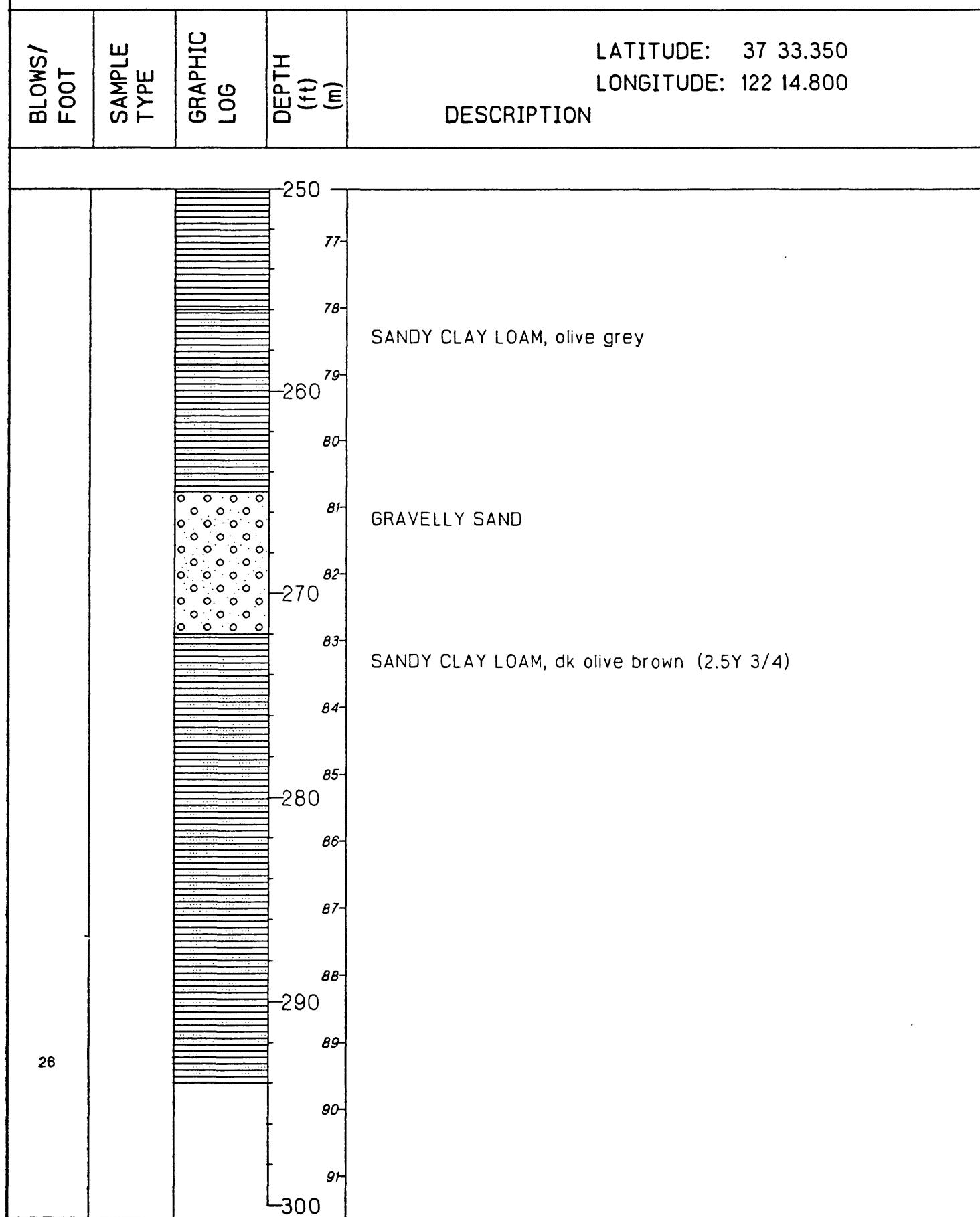
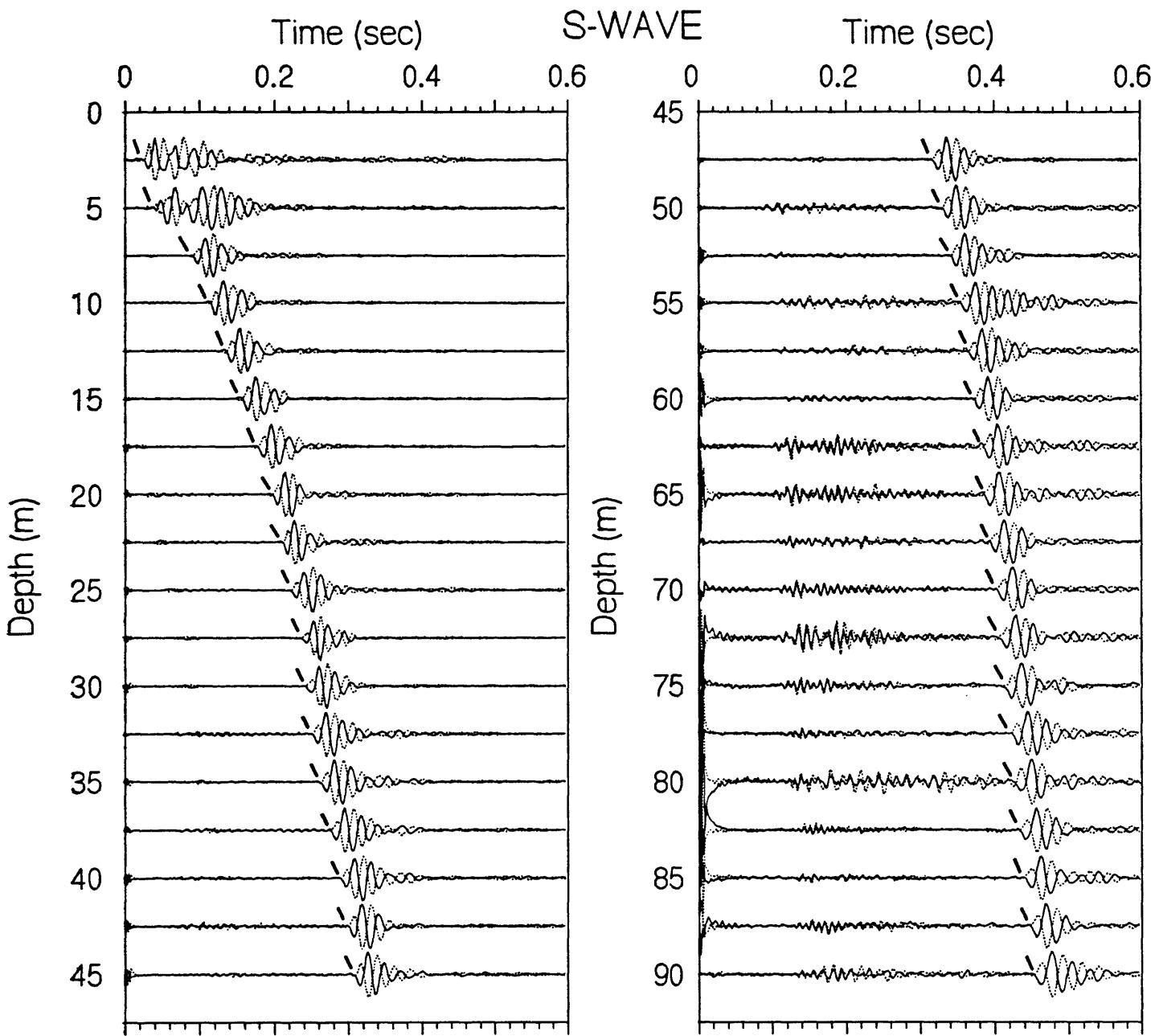
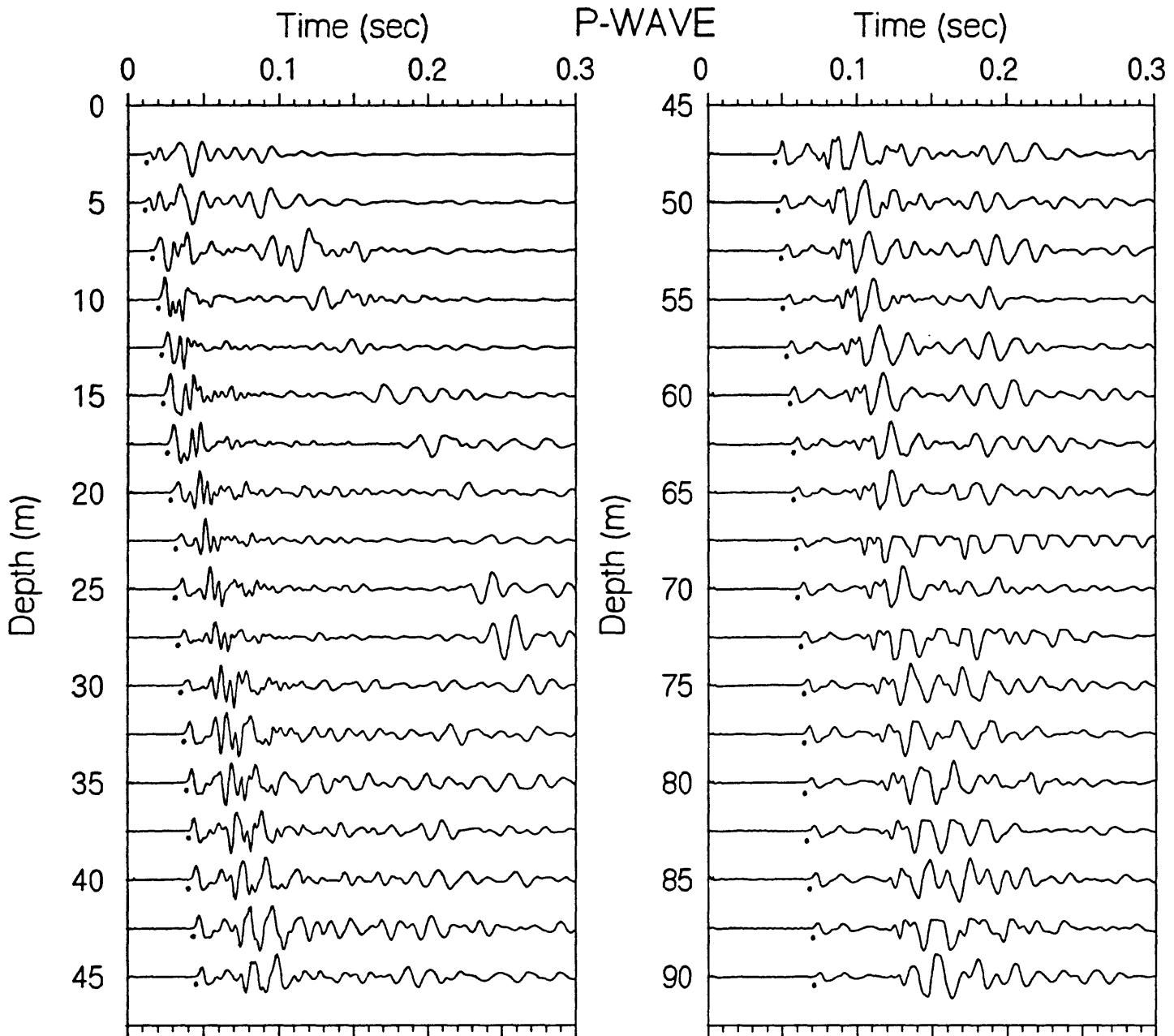


Figure 9. (Continued).



Beach Park Boulevard (Foster City)

Figure 10. Record section of S-waves from impacts in opposite horizontal directions superimposed for identification of S-wave onset. Approximate S-wave picks are shown by the accent marks.



Beach Park Boulevard
(Foster City)

Figure 11. Vertical component record section. P-wave arrivals are shown by the solid circles.

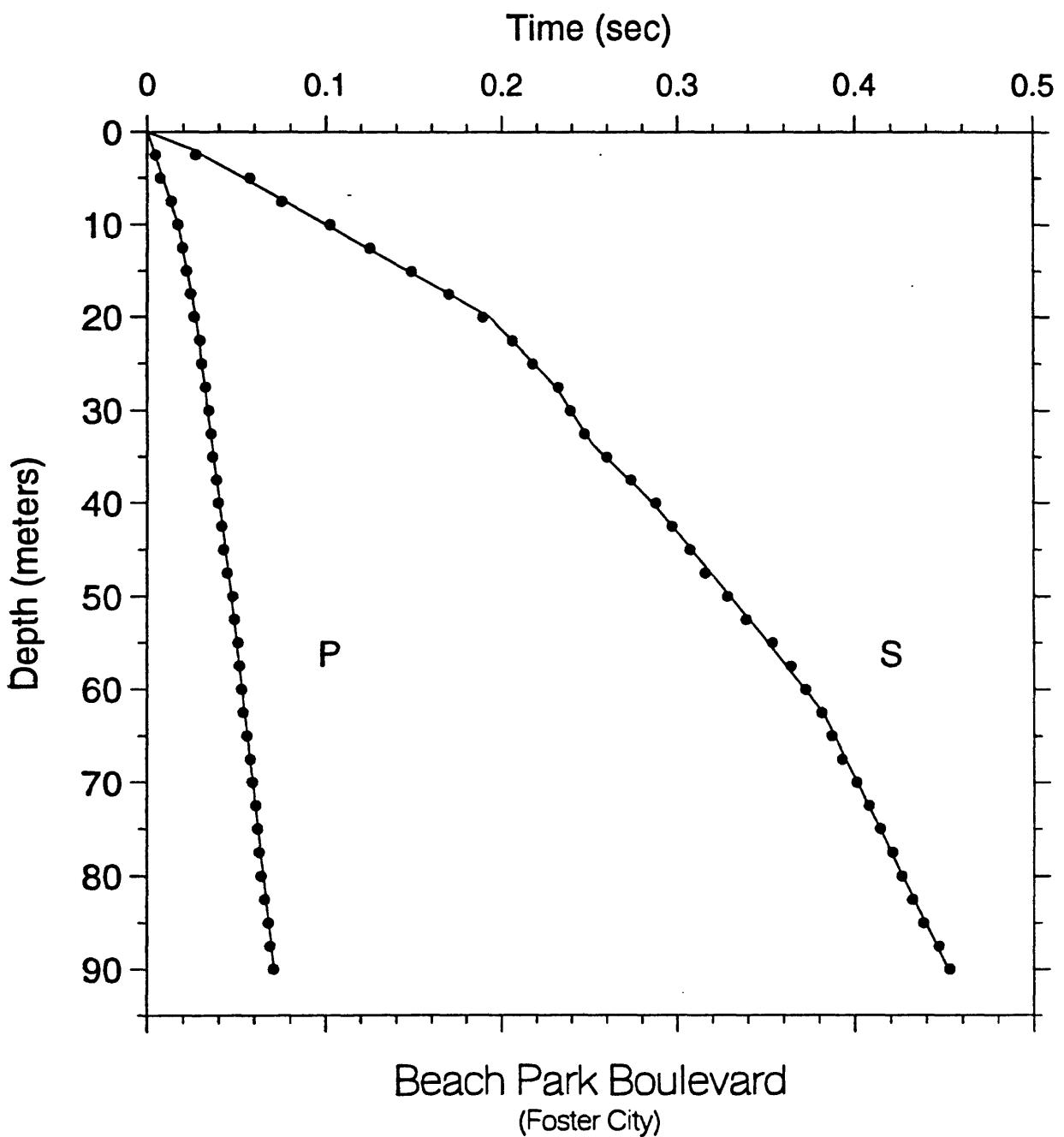


Figure 12. Time-depth graph of P-wave and S-wave picks. Line segments show the hinged-least-squares fit to the data points.

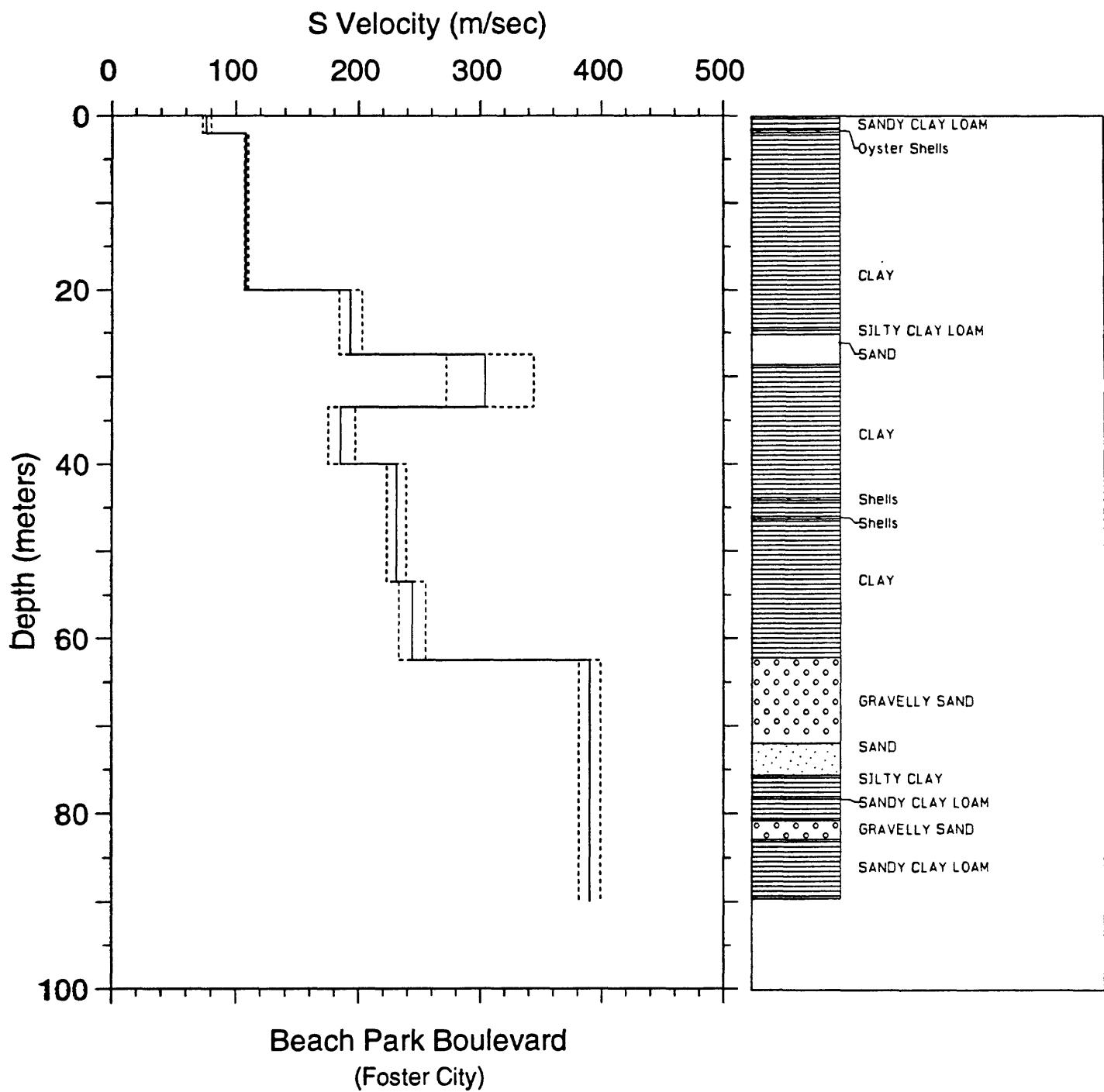


Figure 13. S-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

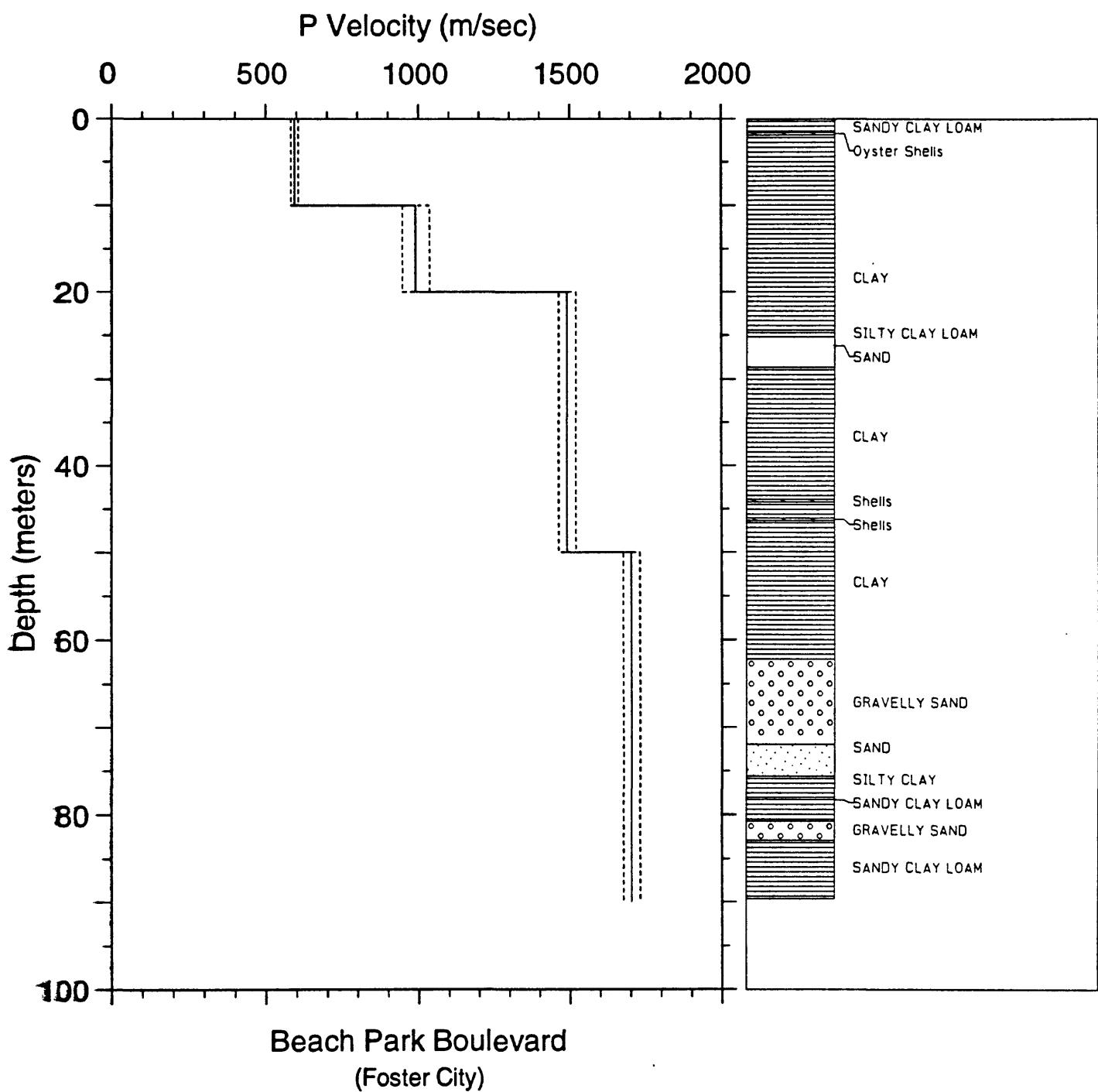


Figure 14. P-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

TABLE 1. S-wave arrival times and velocity summaries for Beach Park Boulevard (Foster City) site.

d(m)	d(ft)	t(sec)	sig	rsdl/sig	dtb(m)	dtb(ft)	ttb(s)	v(m/s)	vl(m/s)	vu(m/s)	v(f/s)	vl(f/s)	vu(f/s)
2.5	8.2	.0270	1	-4.0	.0	.0	.000	76	73	80	249	238	261
5.0	16.4	.0573	1	-3.2	2.0	6.6	.026	76	73	80	249	238	261
7.5	24.6	.0753	1	-1.9	20.0	65.6	.193	108	107	110	355	351	359
10.0	32.8	.1024	1	2.2	27.5	90.2	.231	193	184	203	634	604	667
12.5	41.0	.1249	1	1.6	33.5	109.9	.251	304	272	344	998	893	1130
15.0	49.2	.1485	1	2.1	40.0	131.2	.286	185	175	197	608	573	648
17.5	57.4	.1697	1	-3.2	53.5	175.5	.345	231	223	239	757	733	783
20.0	65.6	.1892	1	-3.4	62.5	205.1	.382	244	233	255	799	766	835
22.5	73.8	.2060	1	-4	90.0	295.3	.452	390	381	399	1278	1250	1308
25.0	82.0	.2177	1	-8									
27.5	90.2	.2322	1	-8									
30.0	98.4	.2392	1	-5									
32.5	106.6	.2471	1	-8									
35.0	114.8	.2599	1	-6									
37.5	123.0	.2736	1	-9									
40.0	131.2	.2878	1	1.6									
42.5	139.4	.2970	1	-1									
45.0	147.6	.3071	1	-8									
47.5	155.8	.3158	1	-2.9									
50.0	164.0	.3284	1	-1.2									
52.5	172.2	.3390	1	-1.4									
55.0	180.4	.3536	1	2.7									
57.5	188.6	.3641	1	2.9									
60.0	196.9	.3722	1	-8									
62.5	205.1	.3813	1	-4									
65.0	213.3	.3869	1	-1.2									
67.5	221.5	.3929	1	-1.6									
70.0	229.7	.4010	1	-1									
72.5	237.9	.4080	1	-6									
75.0	246.1	.4141	1	-3									
77.5	254.3	.4211	1	-9									
80.0	262.5	.4262	1	-4									
82.5	270.7	.4322	1	-8									
85.0	278.9	.4382	1	-1.2									
87.5	287.1	.4468	1	-9									
90.0	295.3	.4528	1	.5									

Explanation:

dt(m) = depth in meters

dt(ft) = depth in feet

t(sec) = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)

sig = sigma, standard deviation normalized to the standard deviation of best picks

rsdl/sig = least-squares residual divided by sigma

dtb(m) = depth to bottom of layer in meters

dtb(ft) = depth to bottom of layer in feet

ttb(s) = arrival time in seconds to bottom of layer

vl(m/s) = lower limit of velocity in meters per second *

vu(m/s) = upper limit of velocity in meters per second

v(f/s) = velocity in feet per second

vl(f/s) = lower limit of velocity in feet per second

vu(f/s) = upper limit of velocity in feet per second

* see text for explanation of velocity limits

TABLE 2. P-wave arrival times and velocity summaries for Beach Park Boulevard (Foster City) site.

$d(m)$	$d(ft)$	$t(sec)$	sig	$rsdl/sig$	$dtb(m)$	$dtb(ft)$	$ttb(s)$	$v(m/s)$	$vl(m/s)$	$vu(m/s)$	$v(ft/s)$	$vl(ft/s)$	$vu(ft/s)$
2.5	8.2	.0045	1	.3	.0	.0	.000	594	582	606	1947	1908	1988
5.0	16.4	.0071	1	-1.3	10.0	32.8	.0117	594	582	606	1947	1908	1988
7.5	24.6	.0133	1	.7	20.0	65.6	.027	990	947	1037	3248	3105	3403
10.0	32.8	.0170	1	-2	50.0	164.0	.047	1491	1464	1520	4893	4803	4986
12.5	41.0	.0195	1	-1	90.0	295.3	.071	1703	1676	1731	5586	5498	5678
15.0	49.2	.0218	1	-1									
17.5	57.4	.0240	1	-4									
20.0	65.6	.0262	1	-8									
22.5	73.8	.0293	1	-7									
25.0	82.0	.0304	1	-1									
27.5	90.2	.0325	1	-5									
30.0	98.4	.0345	1	-8									
32.5	106.6	.0356	1	-3									
35.0	114.8	.0366	1	-4									
37.5	123.0	.0387	1	-0									
40.0	131.2	.0397	1	-7									
42.5	139.4	.0417	1	-3									
45.0	147.6	.0427	1	-1.0									
47.5	155.8	.0448	1	-6									
50.0	164.0	.0478	1	-7									
52.5	172.2	.0488	1	-3									
55.0	180.4	.0508	1	-8									
57.5	188.6	.0518	1	-3									
60.0	196.9	.0528	1	-1									
62.5	205.1	.0538	1	-6									
65.0	213.3	.0558	1	-1									
67.5	221.5	.0578	1	-5									
70.0	229.7	.0589	1	-1									
72.5	237.9	.0609	1	-6									
75.0	246.1	.0619	1	-2									
77.5	254.3	.0629	1	-3									
80.0	262.5	.0639	1	-8									
82.5	270.7	.0659	1	-3									
85.0	278.9	.0679	1	-3									
87.5	287.1	.0689	1	-2									
90.0	295.3	.0709	1	-3									

Explanation:

$d(m)$ = depth in meters

$d(ft)$ = depth in feet

$t(sec)$ = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)

sig = sigma, standard deviation normalized to the standard deviation of best picks

$rsdl/sig$ = least-squares residual divided by sigma

$dtb(m)$ = depth to bottom of layer in meters

$dtb(ft)$ = depth to bottom of layer in feet

$ttb(s)$ = arrival time in seconds to bottom of layer

$v(m/s)$ = velocity in meters per second

$vl(m/s)$ = lower limit of velocity in meters per second *

$vu(m/s)$ = upper limit of velocity in meters per second

$v(ft/s)$ = velocity in feet per second

$vl(ft/s)$ = lower limit of velocity in feet per second

$vu(ft/s)$ = upper limit of velocity in feet per second

* see text for explanation of velocity limits

UNITED STATES
DEPARTMENT OF THE INTERIOR
GEOLOGICAL SURVEY

NILES QUADRANGLE
CALIFORNIA—ALAMEDA CO.
7.5 MINUTE SERIES (TOPOGRAPHIC)



SCALE 1:24 000

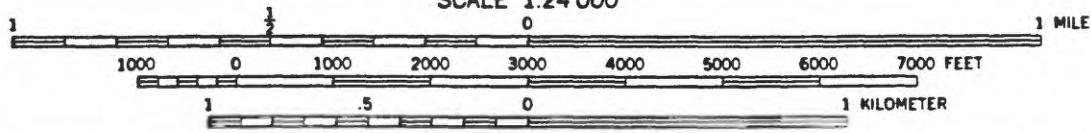


Figure 15. Site location map for Fremont borehole.

Definitions of terms used for descriptions of sedimentary deposits and bedrock materials

Rock hardness: response to hand and geologic hammer: (Ellen et al., 1972)

hard - hammer bounces off with solid sound
 firm - hammer dents with thud, pick point dents or penetrates slightly
 soft - pick points penetrates
 friable material can be crumbled into individual grains by hand.

Fracture spacing: (Ellen et al., 1972)

cm	in	fracture spacing
0-1	0-1/2	v. close
1-5	1/2-2	close
5-30	2-12	moderate
30-100	12-36	wide
> 100	> 36	v. wide

Weathering:

Fresh: no visible signs of weathering

Slight: no visible decomposition of minerals, slight discoloration

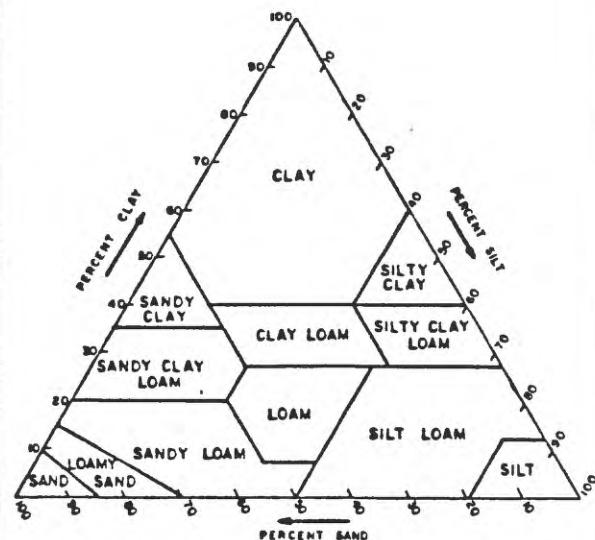
Moderate: slight decomposition of minerals and disintegration of rock, deep and thorough discoloration

Deep: extensive decomposition of minerals and complete disintegration of rock but original structure is preserved.

Relative density of sand and consistency of clay is correlated with penetration resistance: (Terzaghi and Peck, 1948)

blows/ft.	relative density	blows/ft.	consistency
0-4	v. loose	<2	v. soft
4-10	loose	2-4	soft
10-30	medium	4-8	medium
30-50	dense	8-15	stiff
>50	v. dense	15-30	v. stiff
		>30	hard

Texture: the relative proportions of clay, silt, and sand below 2mm. Proportions of larger particles are indicated by modifiers of textural class names. Determination is made in the field mainly by feeling the moist soil (Soil Survey, Staff, 1951).



Color: Standard Munsell color names are given for the dominant color of the moist soil and for prominent mottles.

Types of samples

SP - Standard Penetration 1 + 3/8 in in ID sampler)

S - Thin-wall push sampler

O - Osterberg fixed-piston sampler

P - Pitcher Barrel sampler

CH - California Penetration (2 in ID sampler)

DC - Diamond Core

Figure 16. Explanation of geologic log.

SITE: FREMONT

DATE:

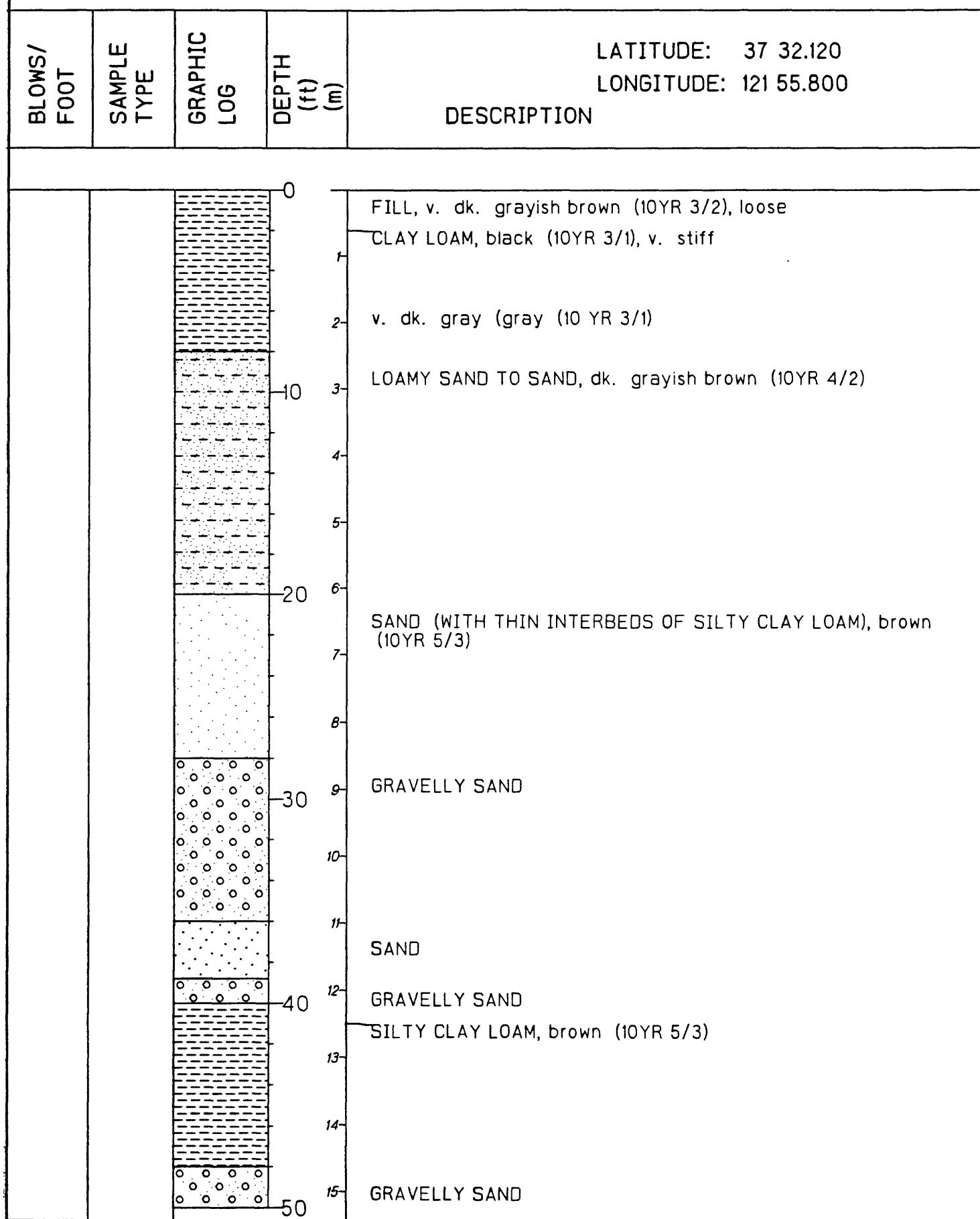


Figure 17. Geologic log of Fremont borehole.

SITE: FREMONT

DATE:

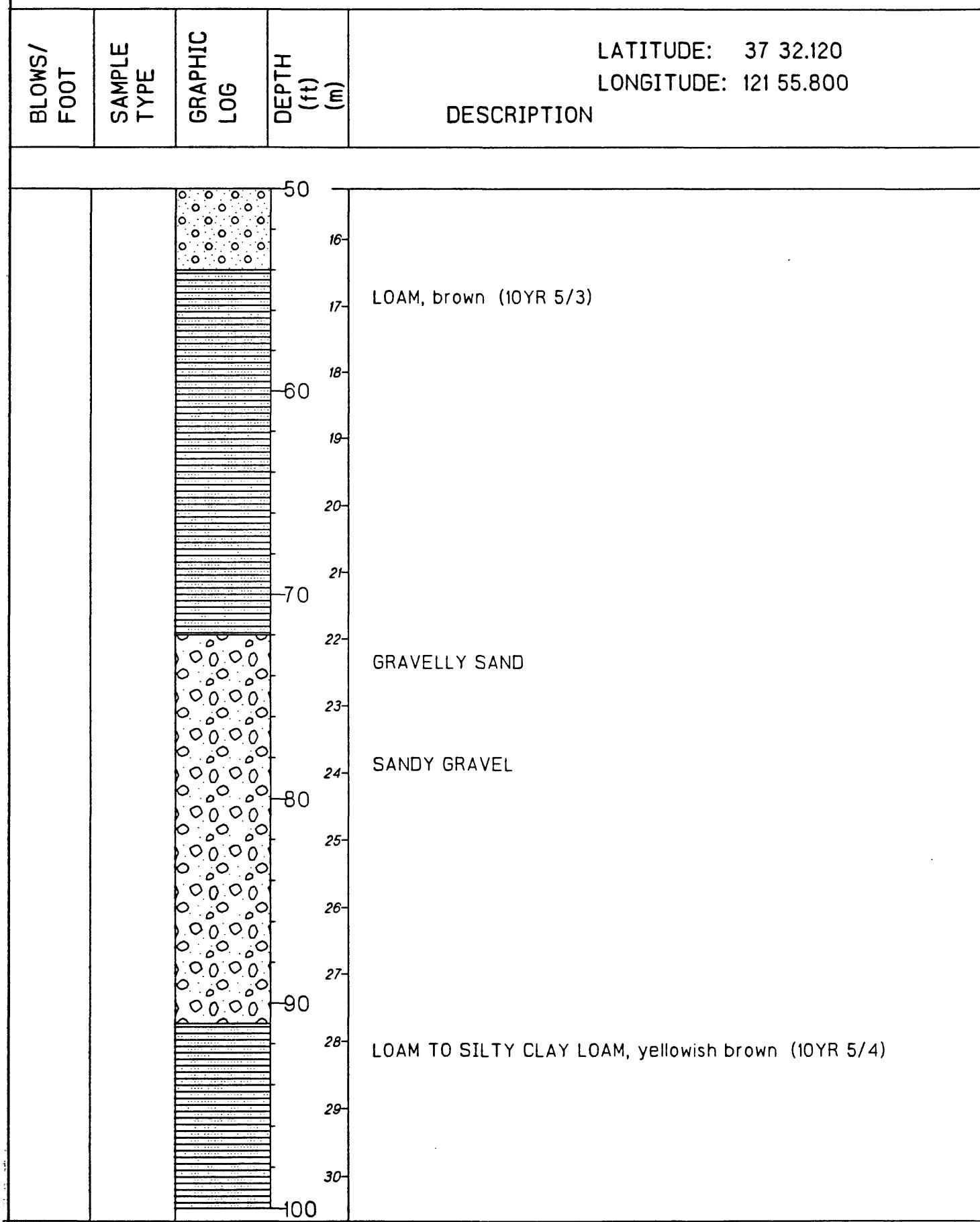


Figure 17. (Continued).

SITE: FREMONT

DATE:

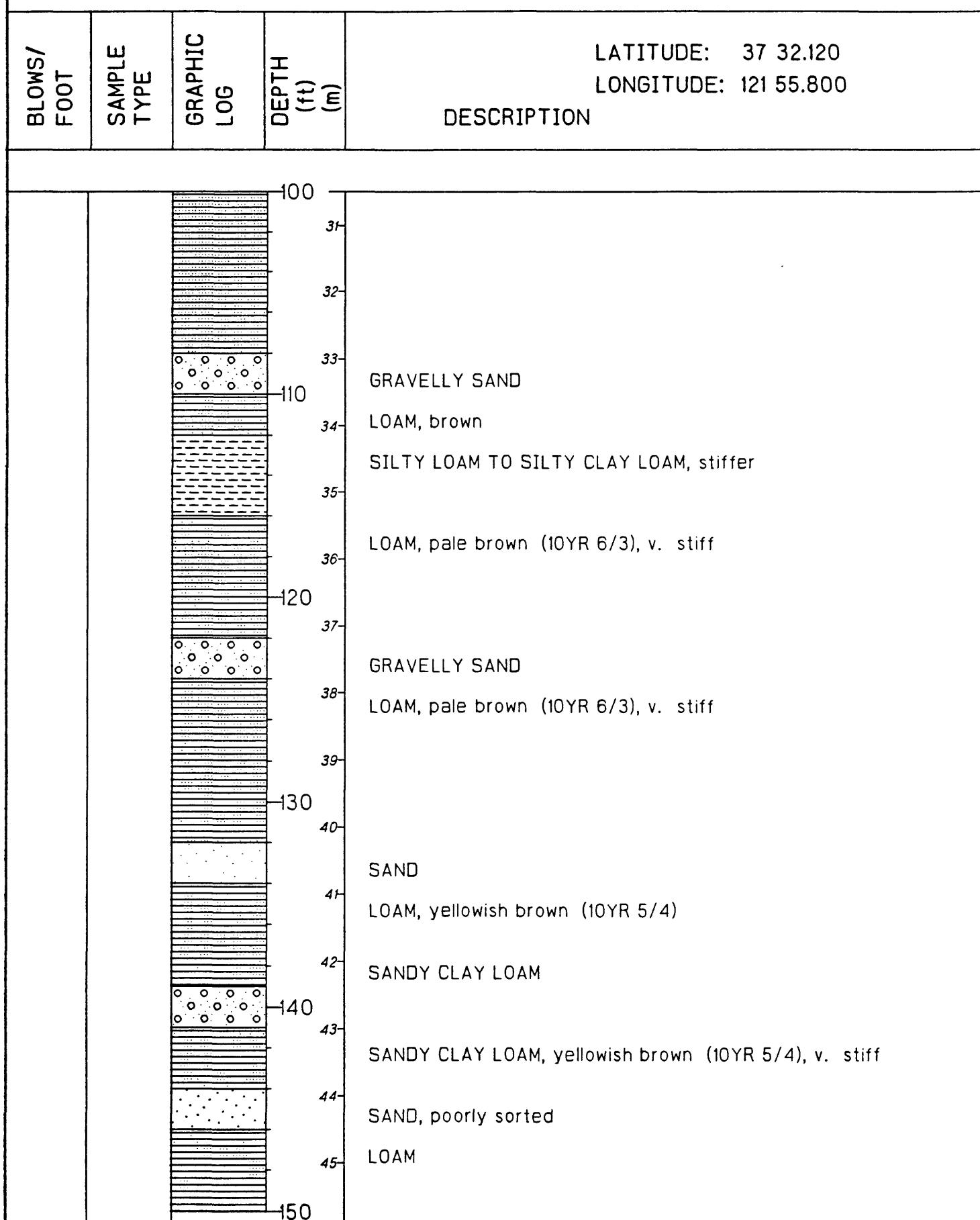


Figure 17. (Continued).

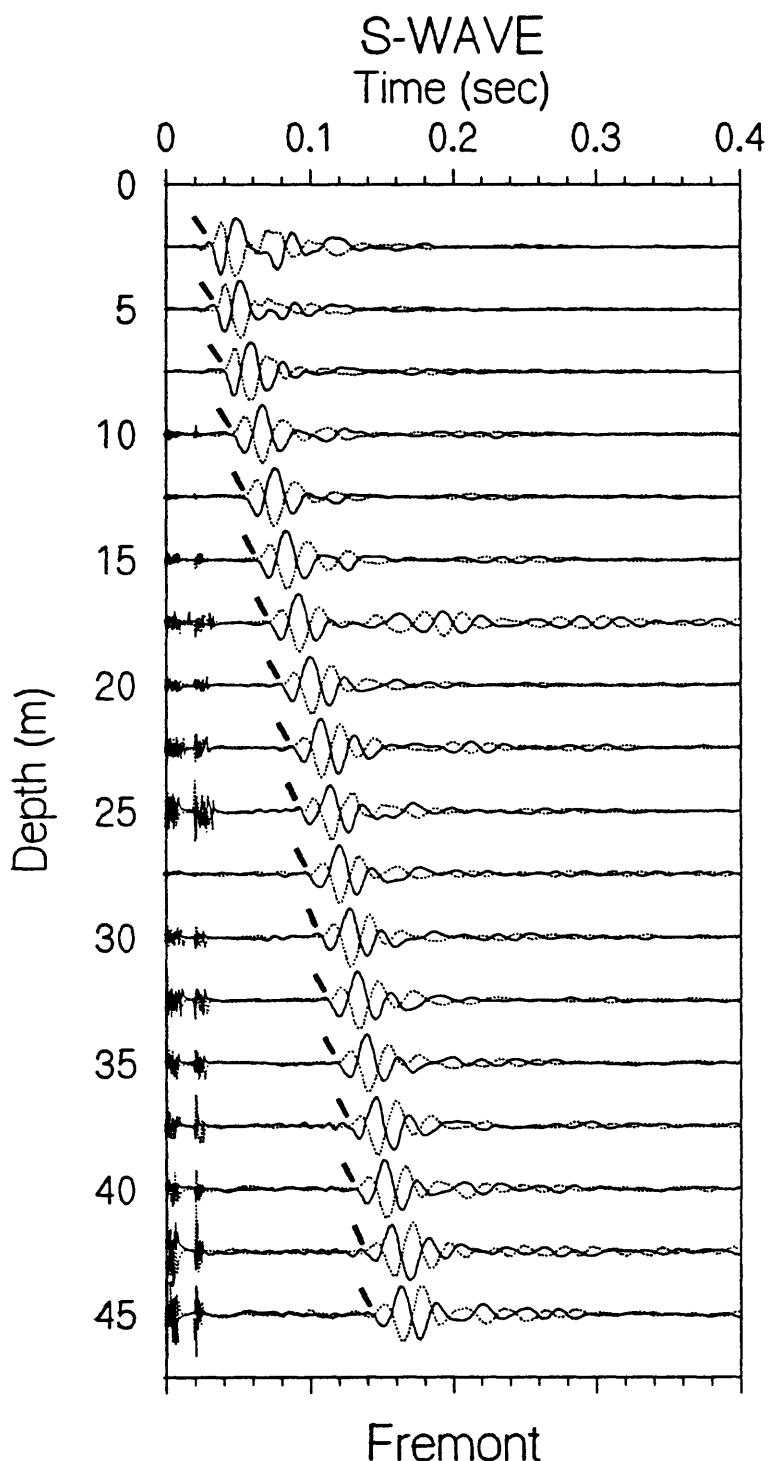


Figure 18. Horizontal-component record section (from horizontal impacts in opposite directions) superimposed for identification of S-wave arrivals. Approximate S-wave picks are shown by the accent marks.

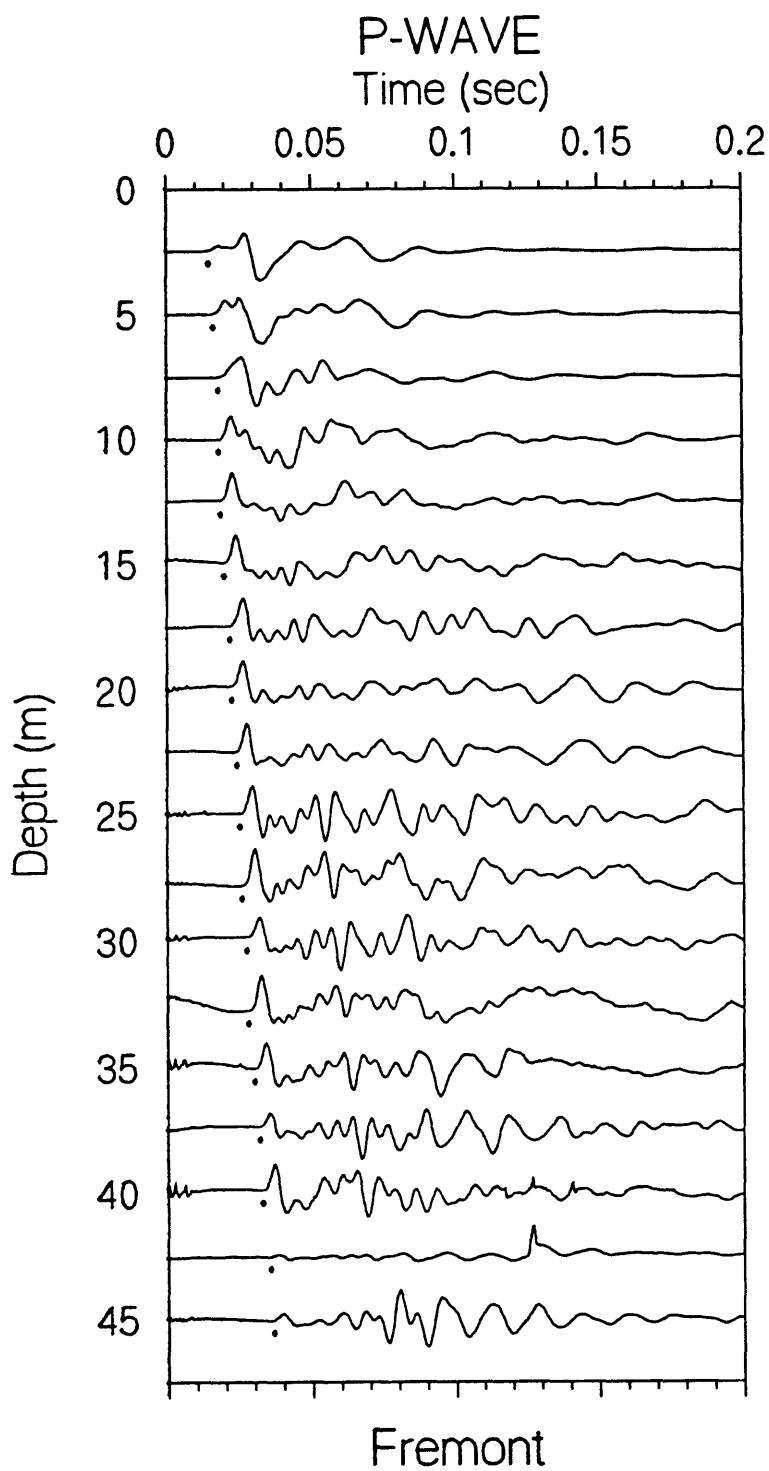


Figure 19. Vertical-component record section. P-waves are shown by the solid circles.

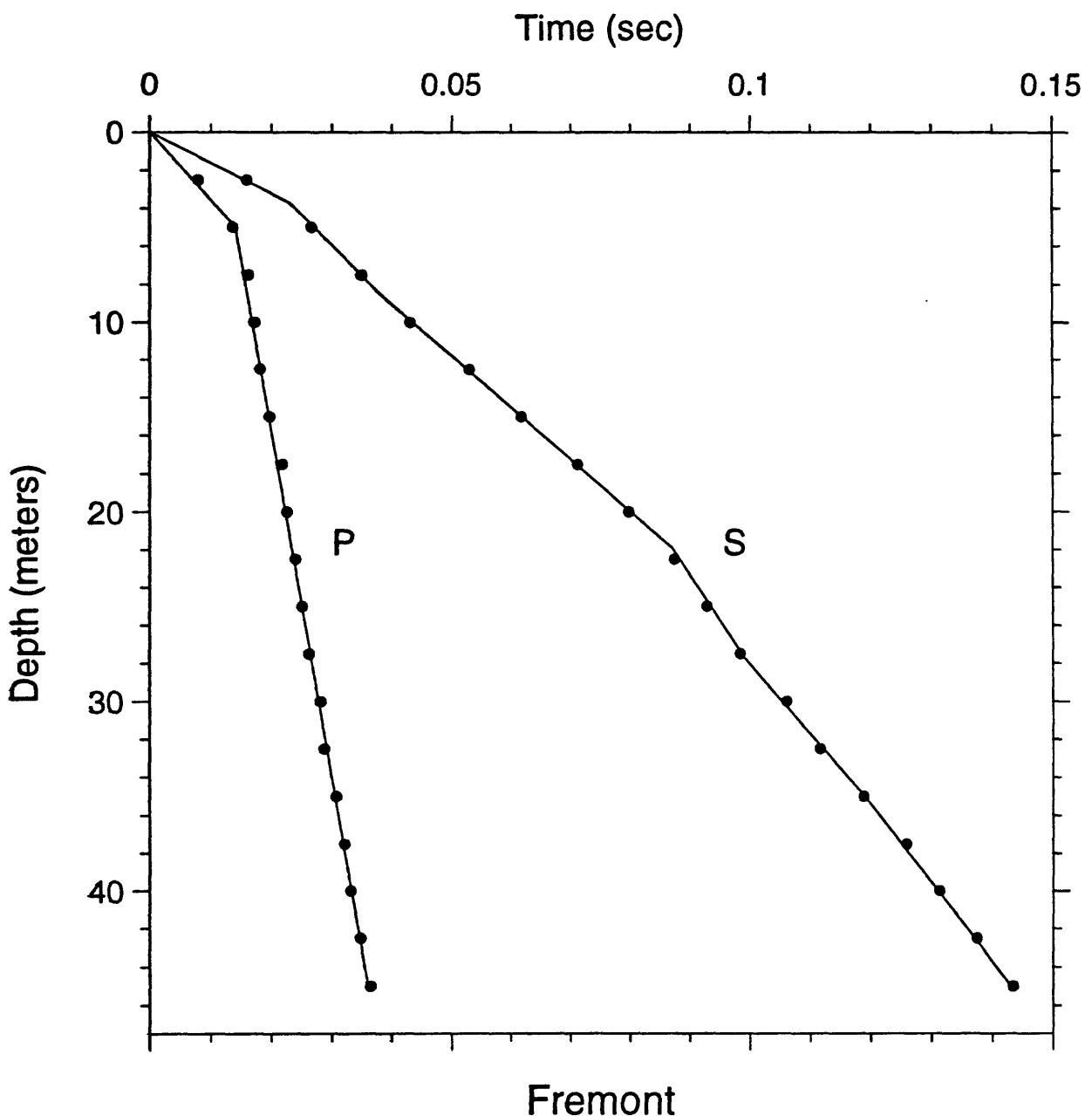


Figure 20. Time-depth graph of P-wave and S-wave picks. Line segments show the hinged-least-squares fit to the data points.

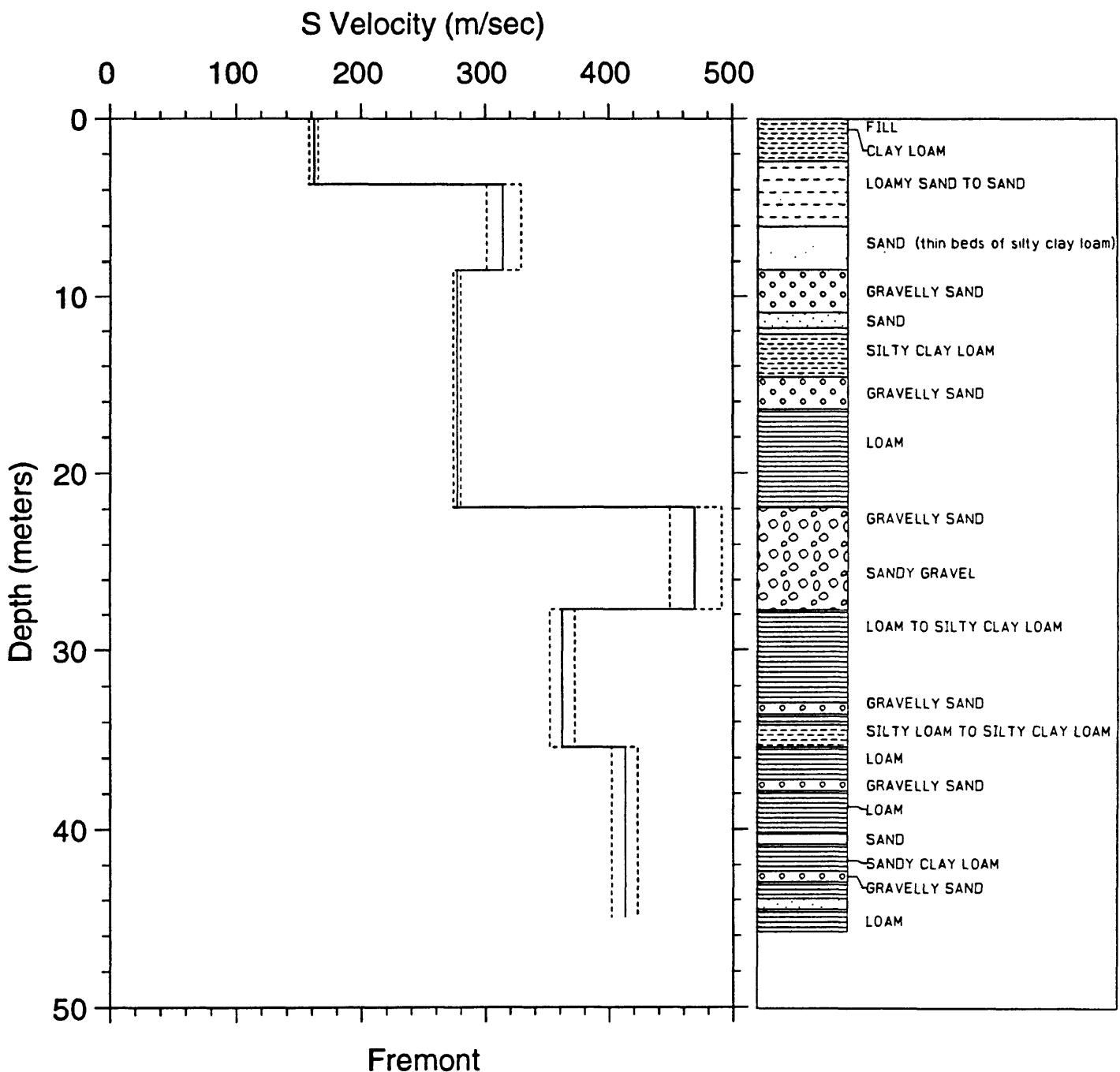


Figure 21. S-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

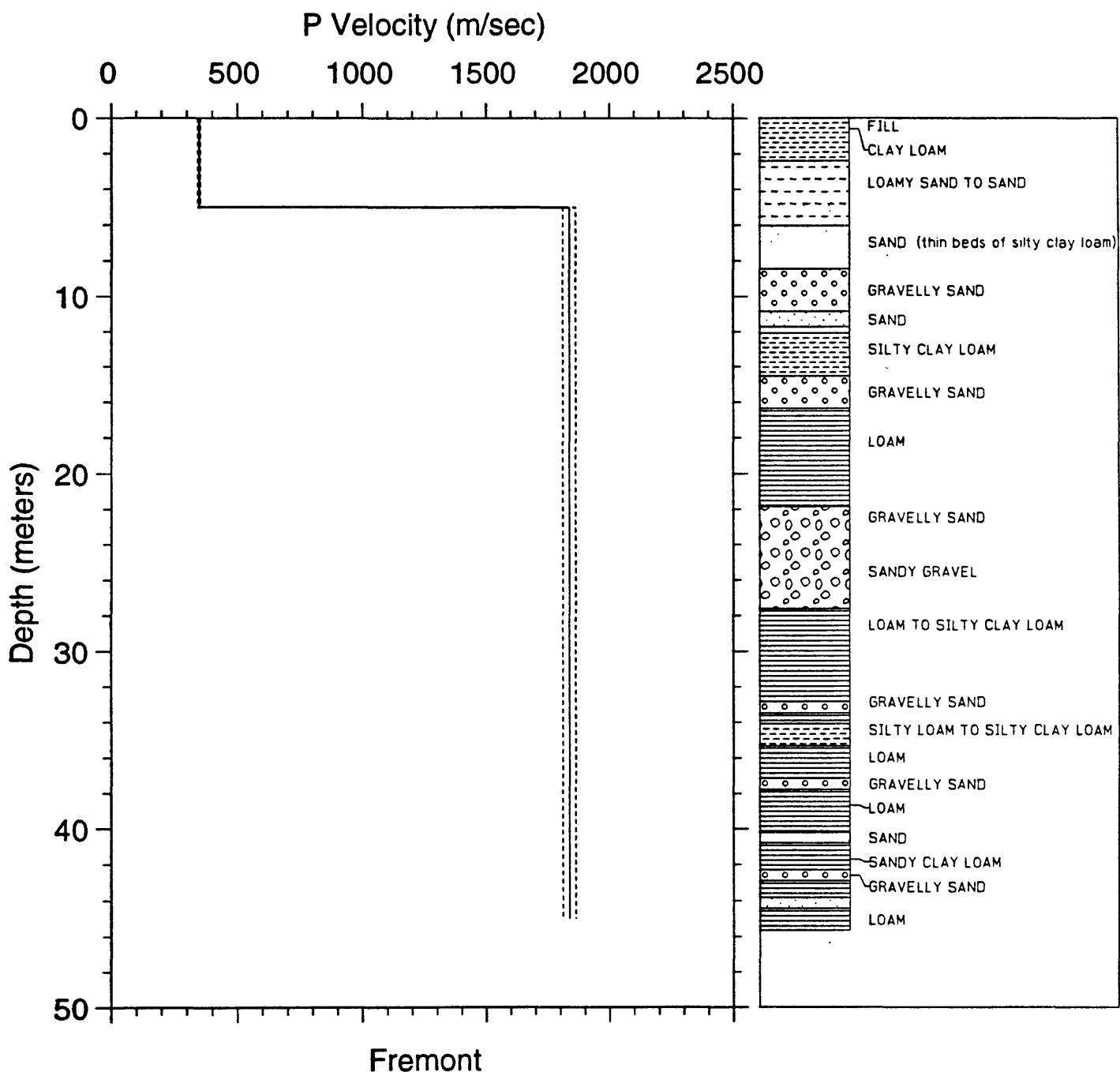


Figure 22. P-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

TABLE 3. S-wave arrival times and velocity summaries for Fremont site.

d(m)	d(ft)	t(sec)	sig	rsdl/sig	dtb(m)	dtb(ft)	ttb(s)	v(m/s)	vl(m/s)	vu(m/s)	v(ft/s)	vl(ft/s)	vu(ft/s)
2.5	8.2	.0159	.4	.4	.0	.0	.000	162	158	165	531	519	542
5.0	16.4	.0266	1	-.4	3.7	12.1	.023	162	158	165	531	519	542
7.5	24.6	.0350	1	-.0	8.5	27.9	.038	314	301	329	1031	987	1079
10.0	32.8	.0431	1	-.5	21.9	71.9	.087	277	274	280	908	898	919
12.5	41.0	.0530	1	-.4	27.7	90.9	.099	469	449	491	1538	1472	1610
15.0	49.2	.0617	1	-.1	35.4	116.1	.120	362	352	372	1186	1155	1219
17.5	57.4	.0712	1	-.5	45.0	147.6	.143	413	402	423	1354	1320	1389
20.0	65.6	.0798	1	-.1									
22.5	73.8	.0874	1	-.4									
25.0	82.0	.0928	1	-.4									
27.5	90.2	.0984	1	-.1									
30.0	98.4	.1061	1	-.8									
32.5	106.6	.1116	1	-.6									
35.0	114.8	.1188	1	-.3									
37.5	123.0	.1259	1	-.6									
40.0	131.2	.1313	1	-.1									
42.5	139.4	.1374	1	-.0									
45.0	147.6	.1434	1	-.1									

Explanation:

d(m) = depth in meters

d(ft) = depth in feet

t(sec) = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)

sig = sigma, standard deviation normalized to the standard deviation of best picks

rsdl/sig = least-squares residual divided by sigma

dtb(m) = depth to bottom of layer in meters

dtb(ft) = depth to bottom of layer in feet

ttb(s) = arrival time in seconds to bottom of layer

v(m/s) = velocity in meters per second

vl(m/s) = lower limit of velocity in meters per second *

vu(m/s) = upper limit of velocity in meters per second

v(ft/s) = velocity in feet per second

vl(ft/s) = lower limit of velocity in feet per second

vu(ft/s) = upper limit of velocity in feet per second

* see text for explanation of velocity limits

TABLE 4. P-wave arrival times and velocity summaries for Fremont site.

d(m)	d(ft)	t(sec)	sig	rsdl/sig	dtb(m)	dtb(ft)	ttb(s)	v(m/s)	vl(m/s)	vu(m/s)	v(ft/s)	vl(ft/s)	vu(ft/s)
2.5	8.2	.0079	1	.7	.0	.0	.000	348	343	352	1141	1127	1155
5.0	16.4	.0136	1	-.8	5.0	16.4	.014	348	343	352	1141	1127	1155
7.5	24.6	.0162	1	-.5	45.0	147.6	.036	1837	1811	1863	6026	5941	6113
10.0	32.8	.0173	1	-.2									
12.5	41.0	.0181	1	-.4									
15.0	49.2	.0197	1	-.1									
17.5	57.4	.0218	1	-.6									
20.0	65.6	.0226	1	-.1									
22.5	73.8	.0240	1	-.1									
25.0	82.0	.0251	1	-.2									
27.5	90.2	.0263	1	-.3									
30.0	98.4	.0282	1	-.2									
32.5	106.6	.0288	1	-.6									
35.0	114.8	.0308	1	-.1									
37.5	123.0	.0322	1	-.1									
40.0	131.2	.0332	1	-.2									
42.5	139.4	.0348	1	-.0									
45.0	147.6	.0365	1	-.3									

Explanation:

d(m) = depth in meters

d(ft) = depth in feet

t(sec) = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)

sig = sigma, standard deviation normalized to the standard deviation of best picks

rsdl/sig = least-squares residual divided by sigma

dtb(m) = depth to bottom of layer in meters

dtb(ft) = depth to bottom of layer in feet

ttb(s) = arrival time in seconds to bottom of layer

v(m/s) = velocity in meters per second

vl(m/s) = lower limit of velocity in meters per second *

vu(m/s) = upper limit of velocity in meters per second

v(ft/s) = velocity in feet per second

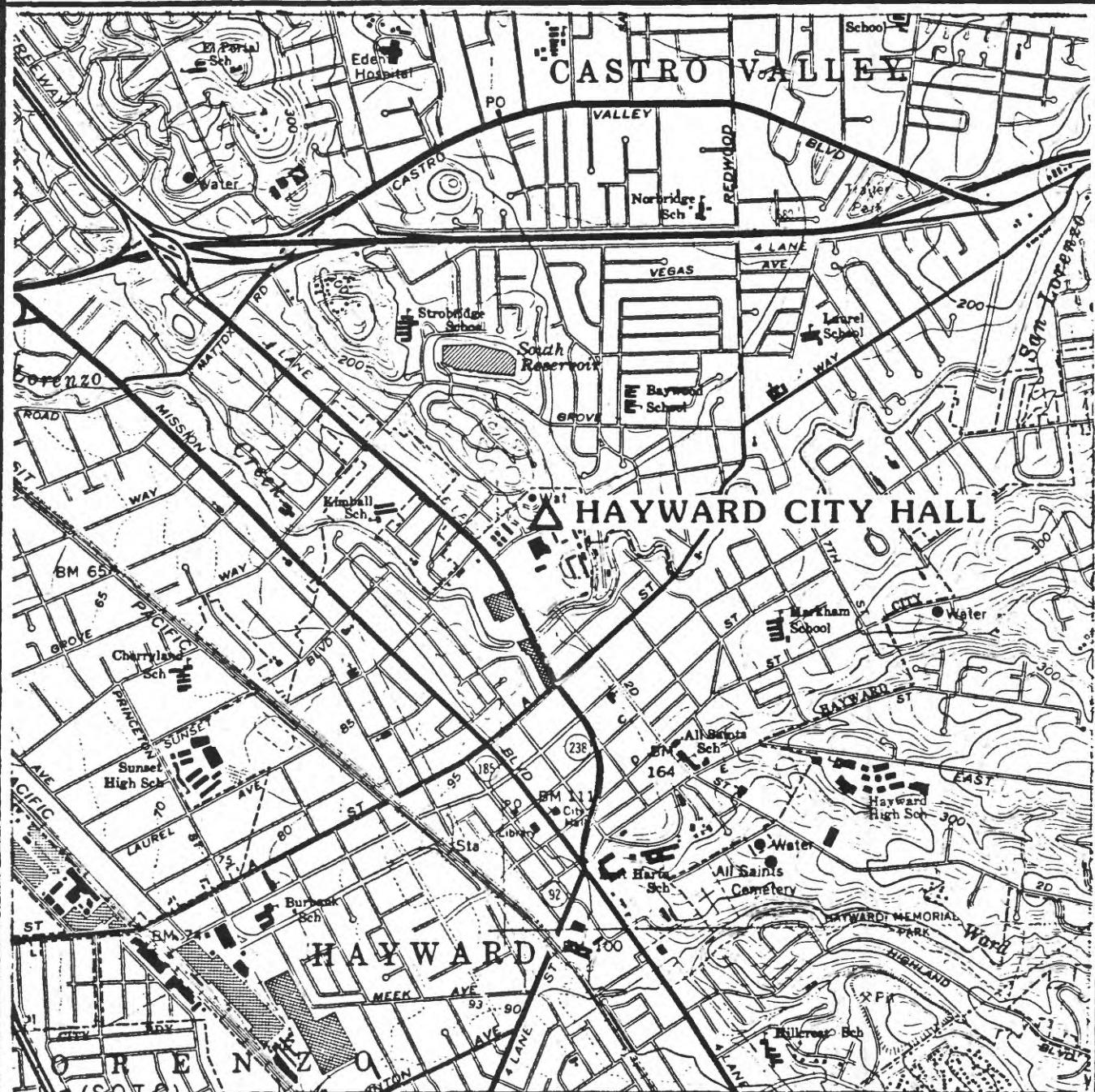
vl(ft/s) = lower limit of velocity in feet per second

vu(ft/s) = upper limit of velocity in feet per second

* see text for explanation of velocity limits

UNITED STATES
DEPARTMENT OF THE INTERIOR
GEOLOGICAL SURVEY

HAYWARD QUADRANGLE
CALIFORNIA
7.5 MINUTE SERIES (TOPOGRAPHIC)
NE 1/4 HAYWARD 15' QUADRANGLE



SCALE 1:24 000

1 MILE

SCALE 1:100,000

1000 0 1000 2000 3000 4000 5000 6000 7000 FEET

1 .5 0 1 KILOMETER

Figure 23. Location map for Hayward City Hall borehole. The borehole is located approximately 100 meters from the strong-motion accelerograph.

Definitions of terms used for descriptions of sedimentary deposits and bedrock materials

Rock hardness: response to hand and geologic hammer: (Ellen et al., 1972)

hard - hammer bounces off with solid sound
 firm - hammer dents with thud, pick point dents or penetrates slightly
 soft - pick points penetrates
 friable material can be crumbled into individual grains by hand.

Fracture spacing: (Ellen et al., 1972)

cm	in	fracture spacing
0-1	0-1/2	v. close
1-5	1/2-2	close
5-30	2-12	moderate
30-100	12-36	wide
>100	>36	v. wide

Weathering:

Fresh: no visible signs of weathering

Slight: no visible decomposition of minerals, slight discoloration

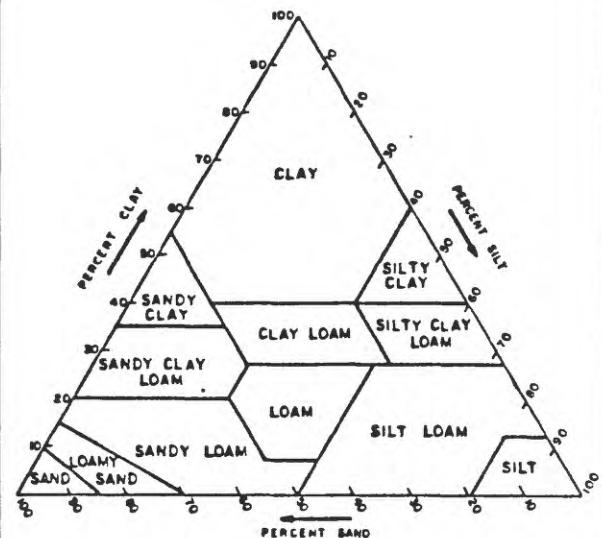
Moderate: slight decomposition of minerals and disintegration of rock, deep and thorough discoloration

Deep: extensive decomposition of minerals and complete disintegration of rock but original structure is preserved.

Relative density of sand and consistency of clay is correlated with penetration resistance: (Terzaghi and Peck, 1948)

blows/ft.	relative density	blows/ft.	consistency
0-4	v. loose	<2	v. soft
4-10	loose	2-4	soft
10-30	medium	4-8	medium
30-50	dense	8-15	stiff
>50	v. dense	15-30	v. stiff
		>30	hard

Texture: the relative proportions of clay, silt, and sand below 2mm. Proportions of larger particles are indicated by modifiers of textural class names. Determination is made in the field mainly by feeling the moist soil (Soil Survey, Staff, 1951).



Color: Standard Munsell color names are given for the dominant color of the moist soil and for prominent mottles.

Types of samples

SP - Standard Penetration 1 + 3/8 in in ID sampler)

S - Thin-wall push sampler

O - Osterberg fixed-piston sampler

P - Pitcher Barrel sampler

CH - California Penetration (2 in ID sampler)

DC - Diamond Core

Figure 24. Explanation of geologic log.

SITE: HAYWARD CITY HALL

DATE: 2/5/91

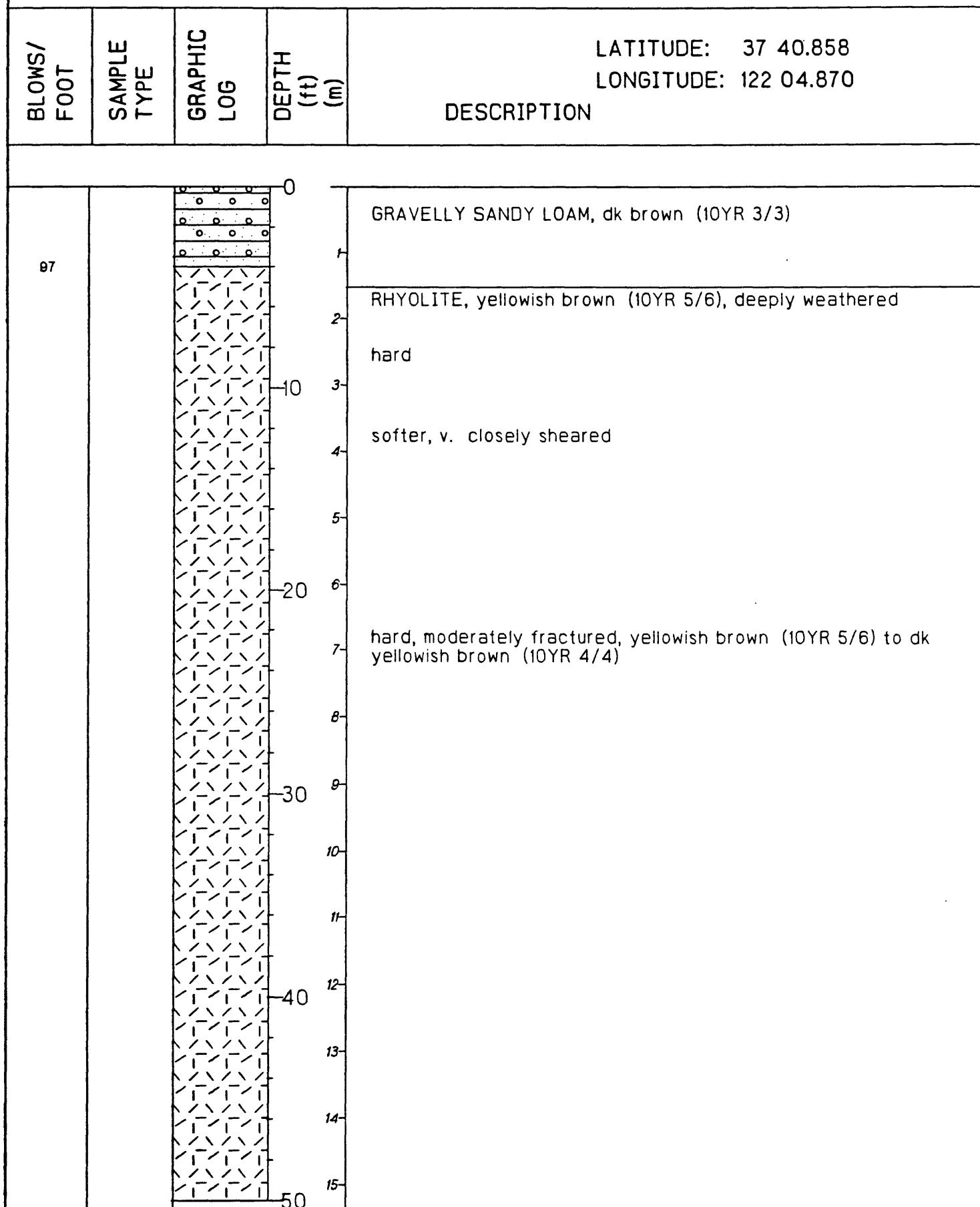


Figure 25. Geologic log of Hayward City Hall borehole.

SITE: HAYWARD CITY HALL

DATE: 2/5/91

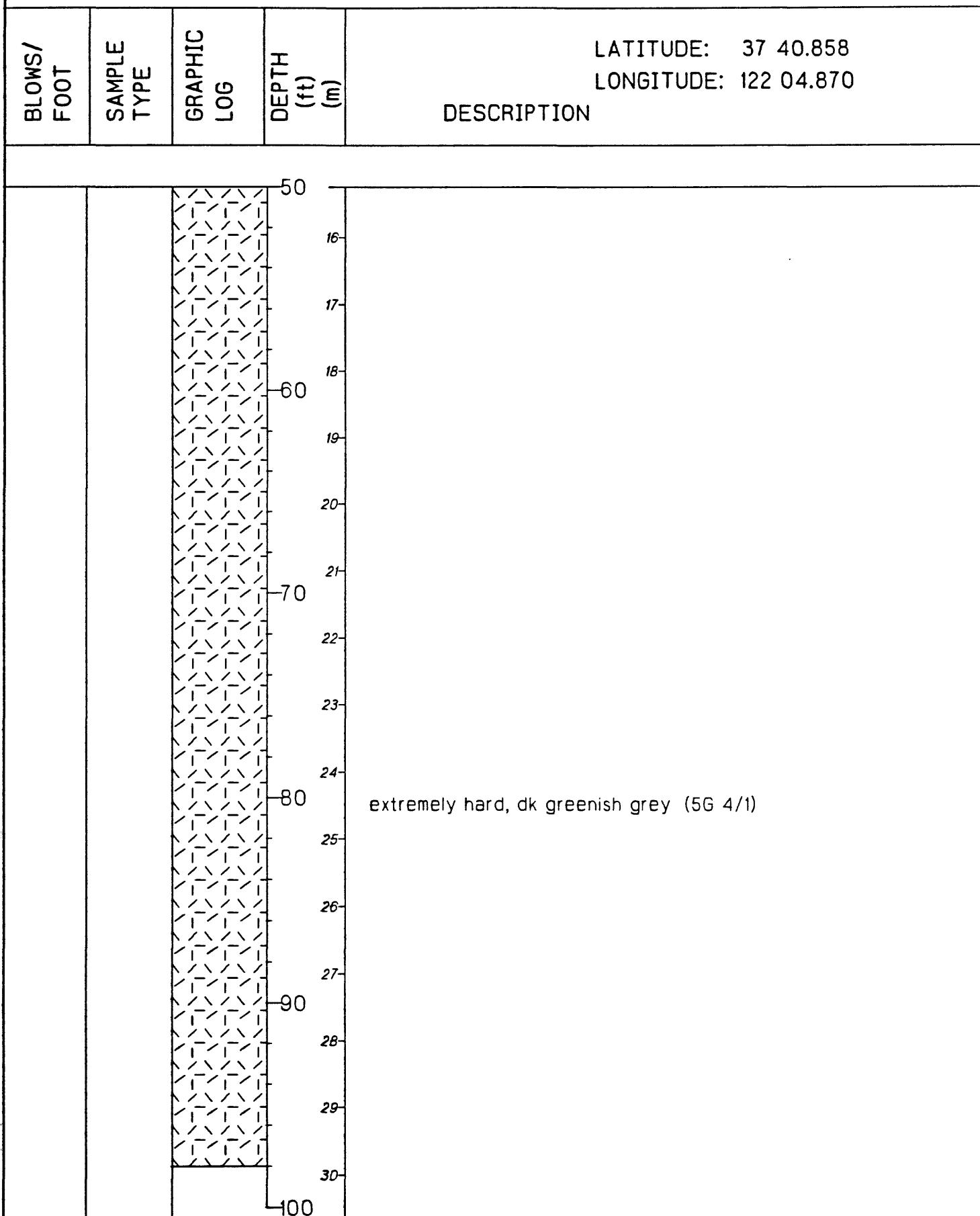
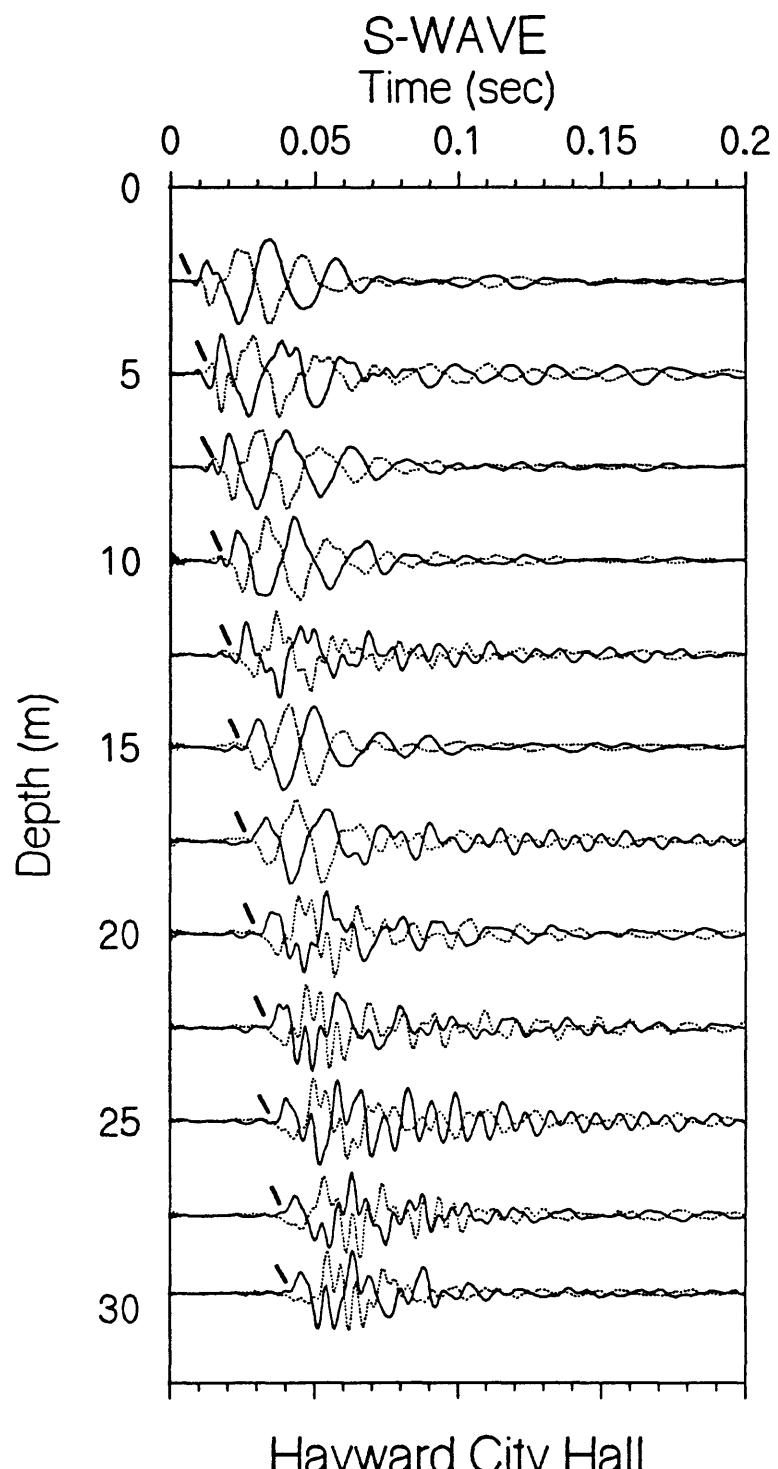


Figure 25. (Continued).



Hayward City Hall

Figure 26. Horizontal-component record section (from horizontal impacts in opposite directions) superimposed for identification of S-wave onset. Approximate S-wave picks are indicated by the accent marks.

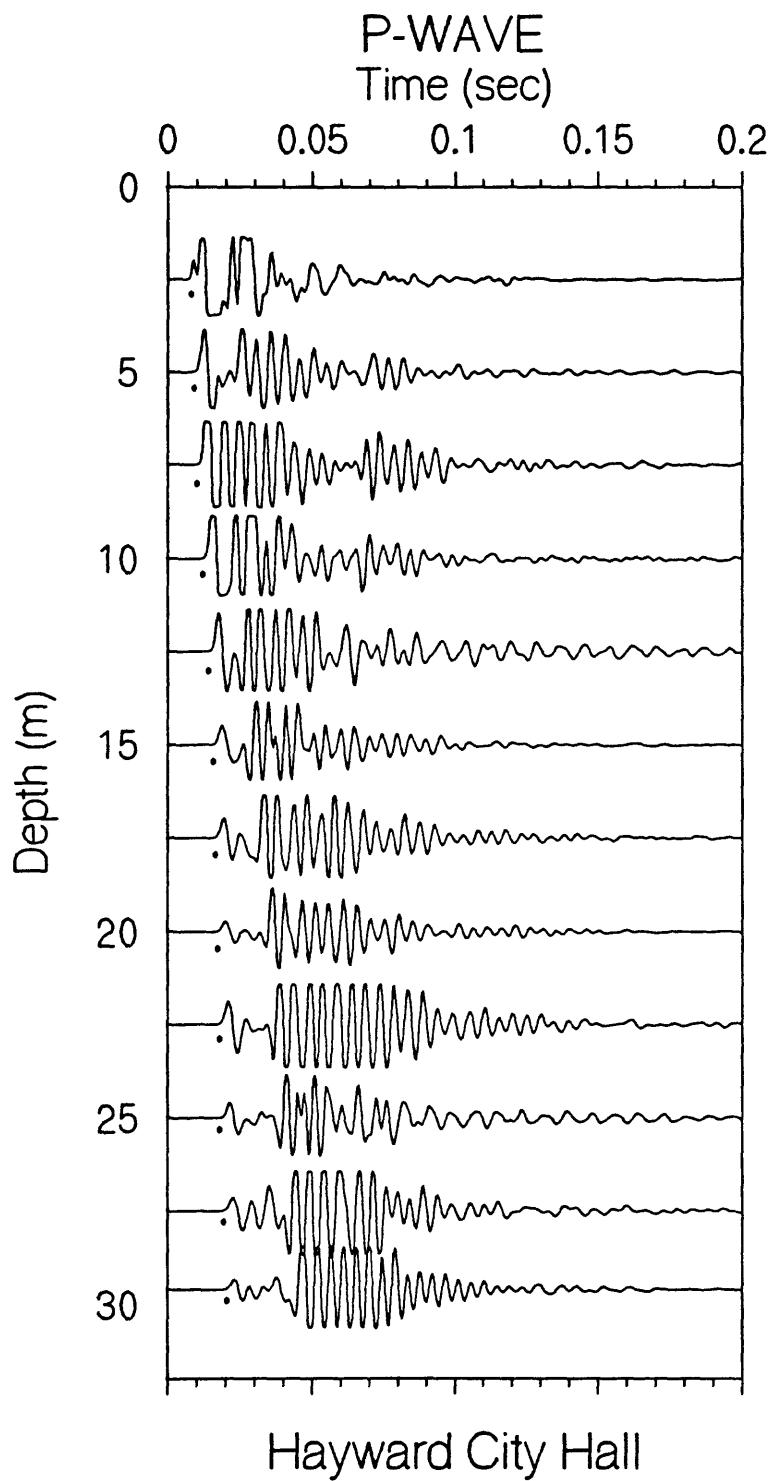


Figure 27. P-wave record section. Approximate P-wave picks are shown by the dots.

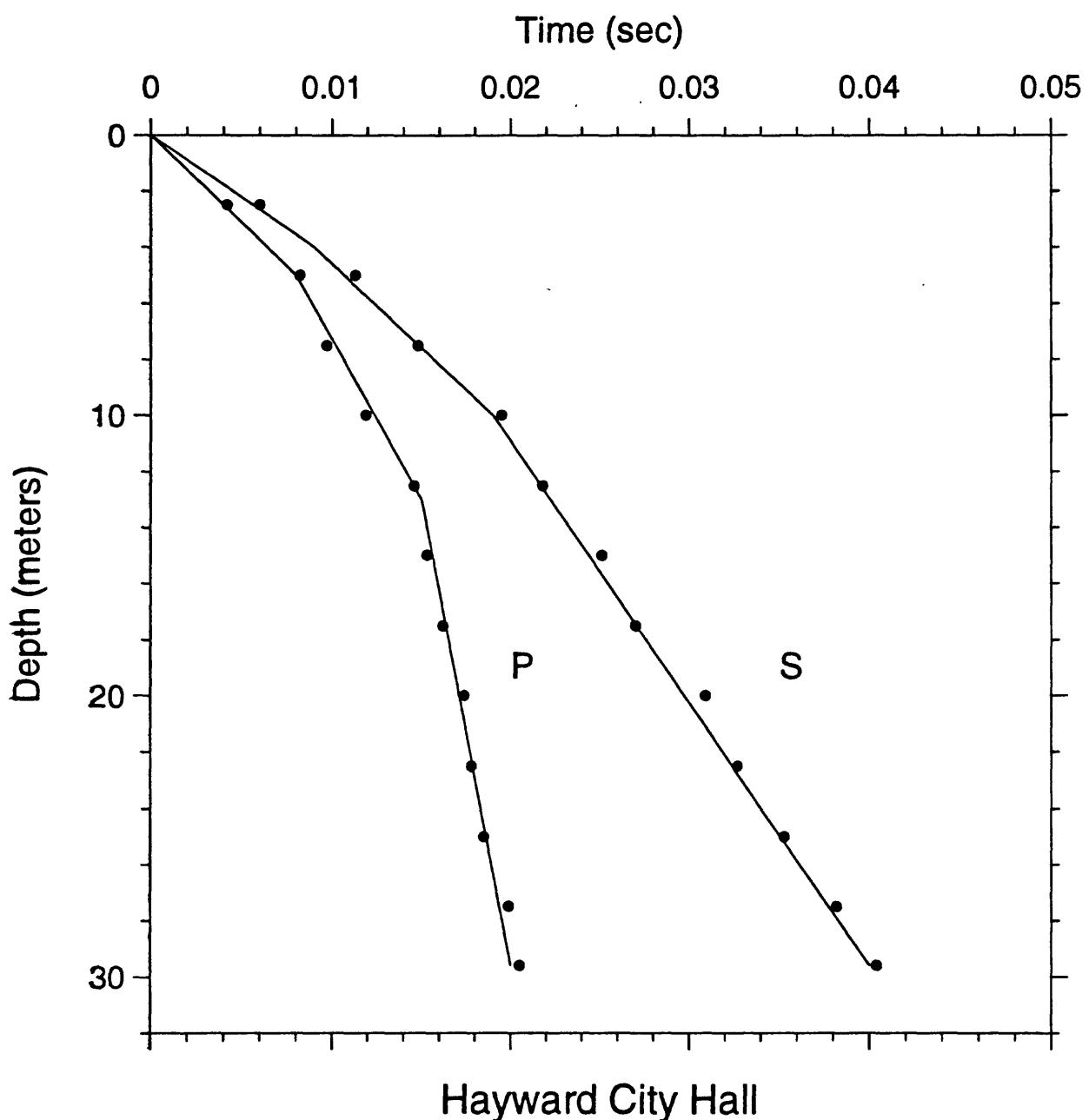


Figure 28. Time-depth graph of P-wave and S-wave picks. Line segments show the hinge-least-squares fit to the data.

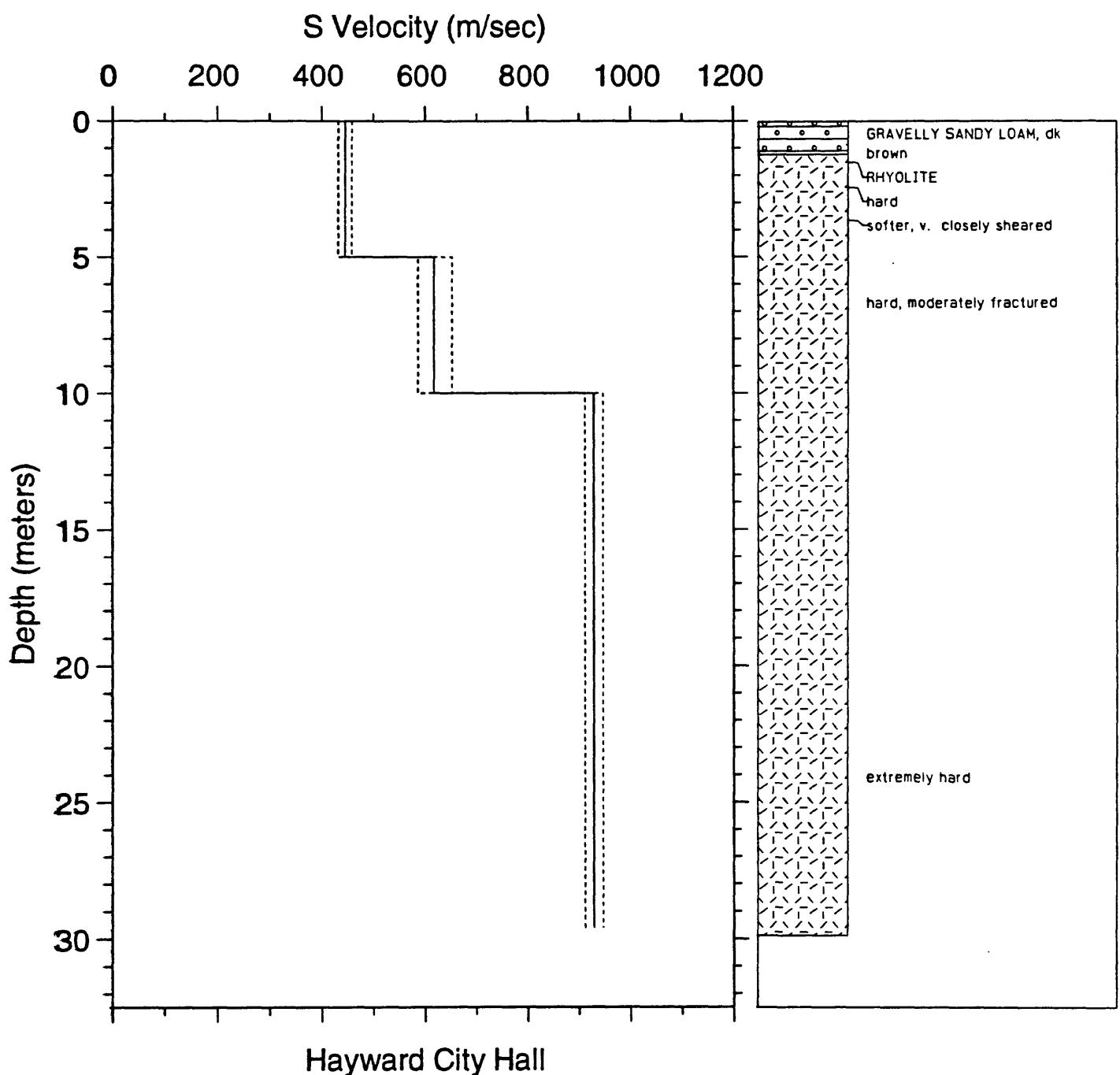


Figure 29. S-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

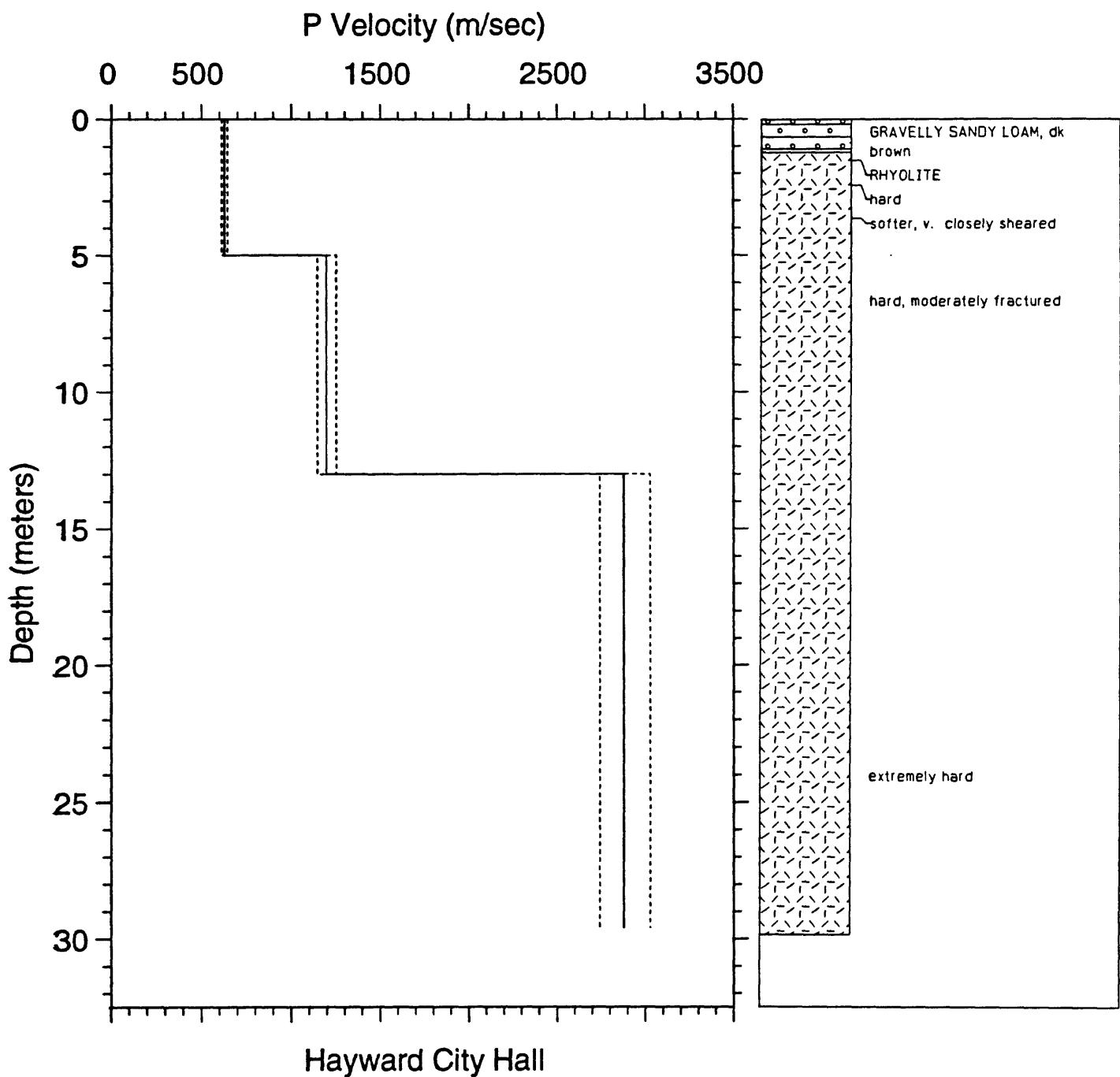


Figure 30. P-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

TABLE 5. S-wave arrival times and velocity summaries for Hayward City Hall site.

d(m)	d(ft)	t(sec)	sig	rsdl/sig	dtb(m)	dtb(ft)	ttb(t)	v(m/s)	v(lm/s)	vu(m/s)	v(f/s)	vl(f/s)	vu(f/s)
2.5	8.2	.0060	1	.4	.0	.0	.0	.445	.432	.458	1459	1417	1503
5.0	16.4	.0113	1	.1	5.0	16.4	.011	.445	.432	.458	1459	1417	1503
7.5	24.6	.0148	1	-.5	10.0	32.8	.019	.617	.586	.652	2024	1922	2138
10.0	32.8	.0195	1	-.2	29.6	97.1	.040	.928	.911	.946	3045	2988	3104
12.5	41.0	.0218	1	-.2									
15.0	49.2	.0251	1	-.4									
17.5	57.4	.0270	1	-.4									
20.0	65.6	.0309	1	-.8									
22.5	73.8	.0327	1	-.1									
25.0	82.0	.0353	1	-.2									
27.5	90.2	.0382	1	-.0									
29.6	97.1	.0404	1	-.1									

Explanation:

- d(m) = depth in meters
- d(ft) = depth in feet
- t(sec) = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)
- sig = sigma, standard deviation normalized to the standard deviation of best picks
- rsdl/sig = least-squares residual divided by sigma
- dtb(m) = depth to bottom of layer in meters
- dtb(ft) = depth to bottom of layer in feet
- ttb(s) = arrival time in seconds to bottom of layer
- v(m/s) = velocity in meters per second
- vl(m/s) = lower limit of velocity in meters per second
- vu(m/s) = upper limit of velocity in meters per second
- v(f/s) = velocity in feet per second
- vl(f/s) = lower limit of velocity in feet per second
- vu(f/s) = upper limit of velocity in feet per second
- * see text for explanation of velocity limits

TABLE 6. P-wave arrival times and velocity summaries for Hayward City Hall site.

d(m)	d(ft)	t(sec)	sig	rsdl/sig	dtb(m)	dtb(ft)	ttb(s)	v(m/s)	vl(m/s)	vu(m/s)	v(t/s)	vl(ft/s)	vu(ft/s)
2.5	8.2	.0042	.1	.2	.0	.0	.000	627	611	644	2056	2004	2112
5.0	16.4	.0082	.1	.2	5.0	16.4	.008	627	611	644	2056	2004	2112
7.5	24.6	.0097	.1	.4	13.0	42.7	.015	1195	1145	1251	3922	3756	4103
10.0	32.8	.0119	.1	.3	29.6	97.1	.020	2878	2743	3028	9444	8998	9936
12.5	41.0	.0146	.1	.3									
15.0	49.2	.0153	.1	.1									
17.5	57.4	.0162	.1	.0									
20.0	65.6	.0174	.1	.3									
22.5	73.8	.0178	.1	.2									
25.0	82.0	.0185	.1	.3									
27.5	90.2	.0199	.1	.2									
29.6		.0205	.1	.1									

Explanation:

d(m) = depth in meters

d(ft) = depth in feet

t(sec) = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)

sig = sigma, standard deviation normalized to the standard deviation of best picks

rsdl/sig = least-squares residual divided by sigma

dtb(m) = depth to bottom of layer in meters

dtb(ft) = depth to bottom of layer in feet

ttb(s) = arrival time in seconds to bottom of layer

vl(m/s) = velocity in meters per second

vl(ft/s) = lower limit of velocity in meters per second *

vu(m/s) = upper limit of velocity in meters per second

v(t/s) = velocity in feet per second

vl(ft/s) = lower limit of velocity in feet per second

vu(ft/s) = upper limit of velocity in feet per second

* see text for explanation of velocity limits



SCALE 1:24 000

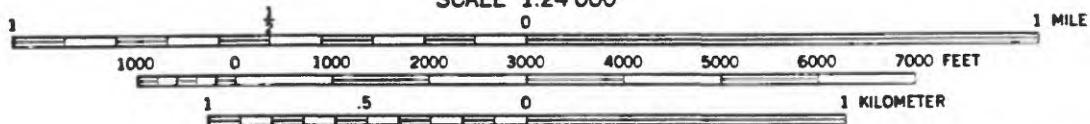


Figure 31. Location map for Larkspur Ferry borehole. The strong-motion accelerograph is located within 10 meters of the borehole.

Definitions of terms used for descriptions of sedimentary deposits and bedrock materials

Rock hardness: response to hand and geologic hammer: (Ellen et al., 1972)

hard - hammer bounces off with solid sound
 firm - hammer dents with thud, pick point dents or penetrates slightly
 soft - pick points penetrates
 friable material can be crumbled into individual grains by hand.

Fracture spacing: (Ellen et al., 1972)

cm	in	fracture spacing
0-1	0-1/2	v. close
1-5	1/2-2	close
5-30	2-12	moderate
30-100	12-36	wide
> 100	> 36	v. wide

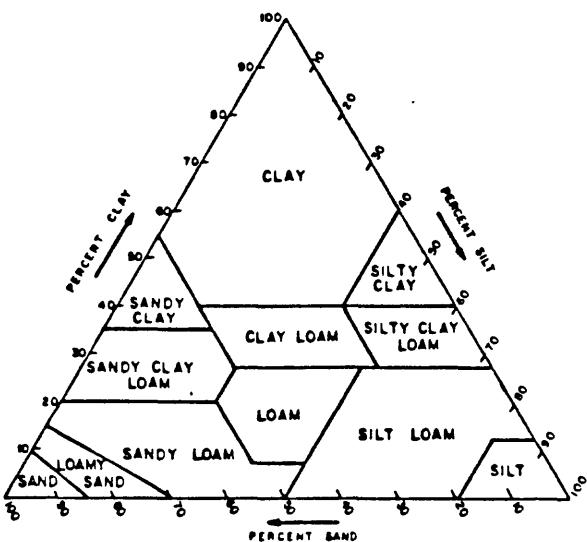
Weathering:

- Fresh: no visible signs of weathering
- Slight: no visible decomposition of minerals, slight discoloration
- Moderate: slight decomposition of minerals and disintegration of rock, deep and thorough discoloration
- Deep: extensive decomposition of minerals and complete disintegration of rock but original structure is preserved.

Relative density of sand and consistency of clay is correlated with penetration resistance: (Terzaghi and Peck, 1948)

blows/ft.	relative density	blows/ft.	consistency
0-4	v. loose	< 2	v. soft
4-10	loose	2-4	soft
10-30	medium	4-8	medium
30-50	dense	8-15	stiff
> 50	v. dense	15-30	v. stiff
		> 30	hard

Texture: the relative proportions of clay, silt, and sand below 2mm. Proportions of larger particles are indicated by modifiers of textural class names. Determination is made in the field mainly by feeling the moist soil (Soil Survey, Staff, 1951).



Color: Standard Munsell color names are given for the dominant color of the moist soil and for prominent mottles.

Types of samples

- SP - Standard Penetration 1 + 3/8 in in ID sampler)
- S - Thin-wall push sampler
- O - Osterberg fixed-piston sampler
- P - Pitcher Barrel sampler
- CH - California Penetration (2 in ID sampler)
- DC - Diamond Core

Figure 32. Explanation of geologic log.

SITE: LARKSPUR FERRY

DATE:

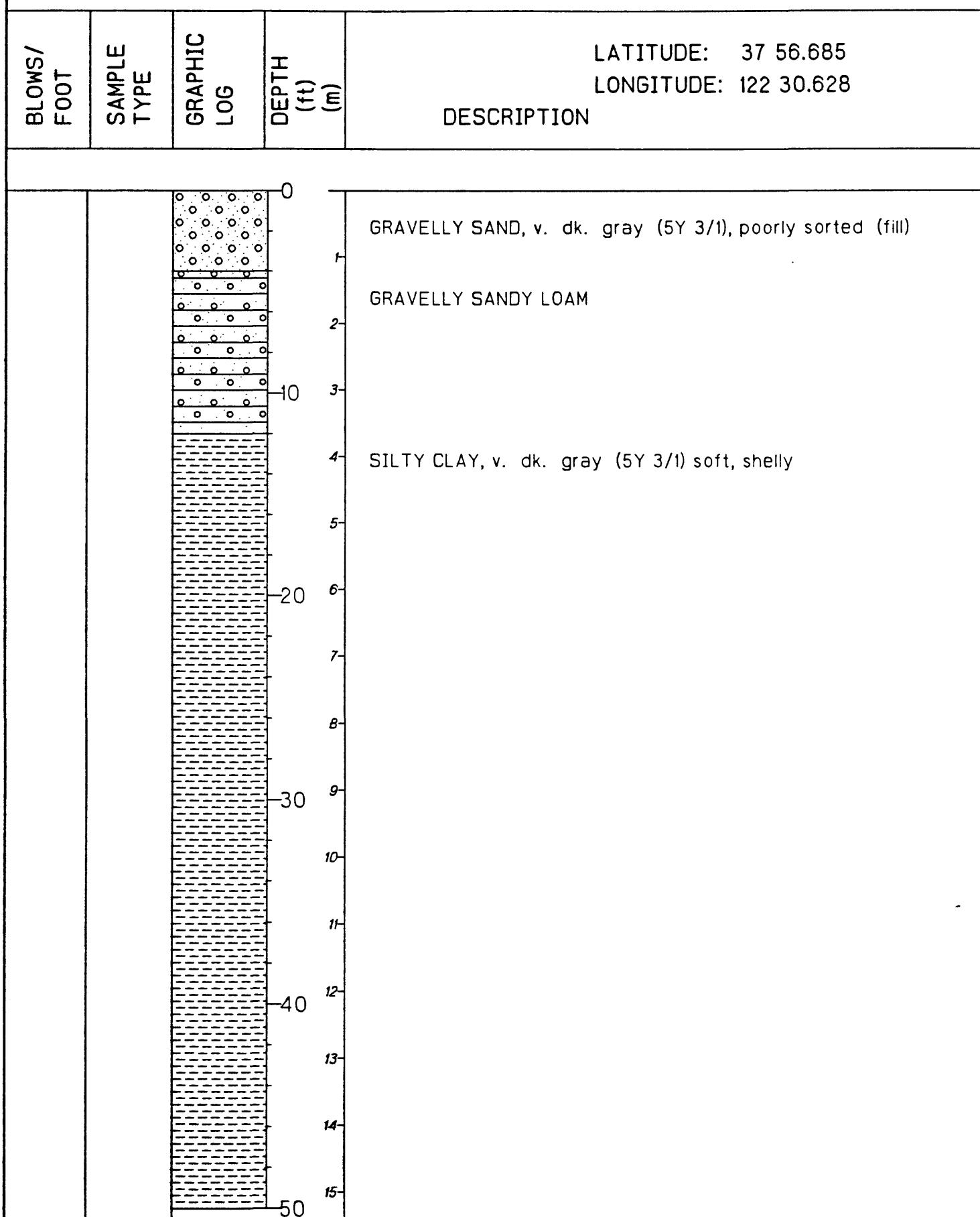


Figure 33. Geologic log of Larkspur Ferry borehole.

SITE: LARKSPUR FERRY

DATE:

BLOWS/ FOOT	SAMPLE TYPE	GRAPHIC LOG	DEPTH (ft) (m)	LATITUDE: 37 56.685 LONGITUDE: 122 30.628 DESCRIPTION
			50	
			16	
			17	
			18	
			60	SANDY CLAY LOAM, dk. grayish brown (10YR 4/2), some gravel
			19	dk. brown (10YR 3/3)
			20	
			21	FINE SANDY CLAY, brownish yellow (10YR 4/6) to yellowish brown (10YR 5/4), stiff
			70	
			22	
			23	COARSE SANDY CLAY, dk. yellowish brown (10YR 4/4)
			24	
			80	
			25	
			26	
			27	
			90	GRAVELLY SANDY LOAM
			28	SANDY LOAM, dense
			29	SHALE (with occasional SANDSTONE interbeds), shale is black, sandstone is v. dk. gray, v. hard
			30	
			100	

Figure 33 (Continued).

SITE: LARKSPUR FERRY

DATE:

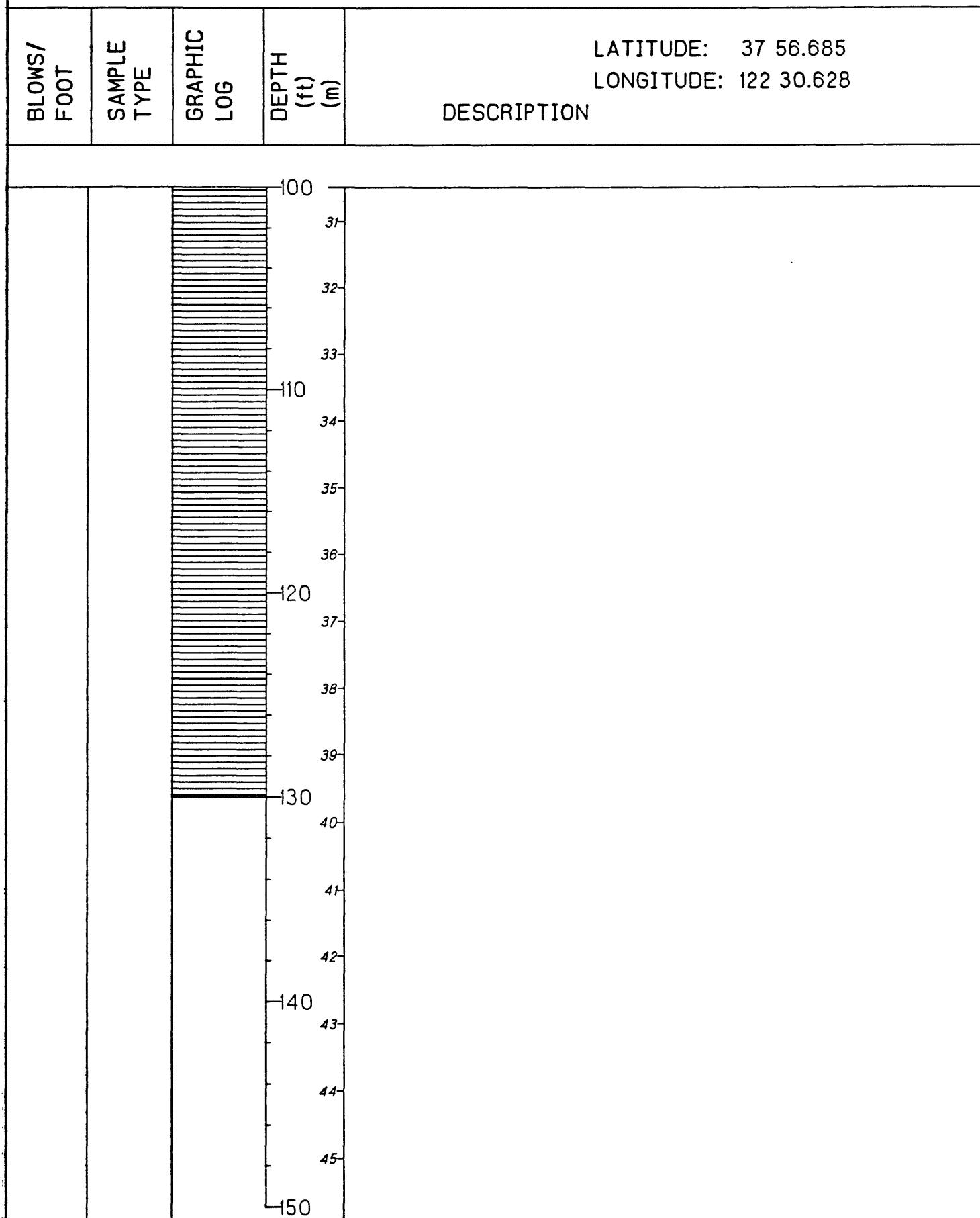
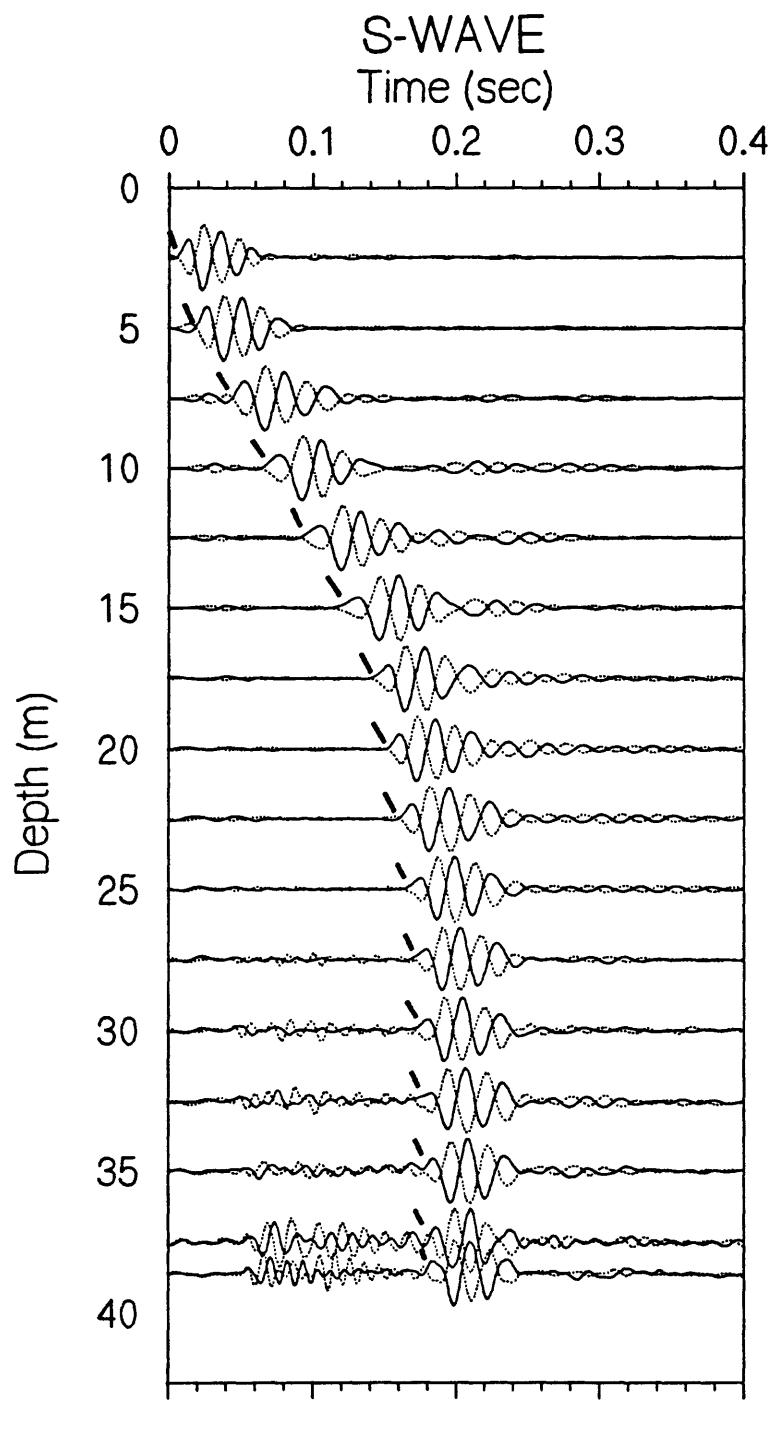


Figure 33 (Continued).



Larkspur Ferry

Figure 34. Horizontal-component record section (from horizontal impacts in opposite directions) superimposed for identification of S-wave onset. Approximate S-wave picks are indicated by the accent marks.

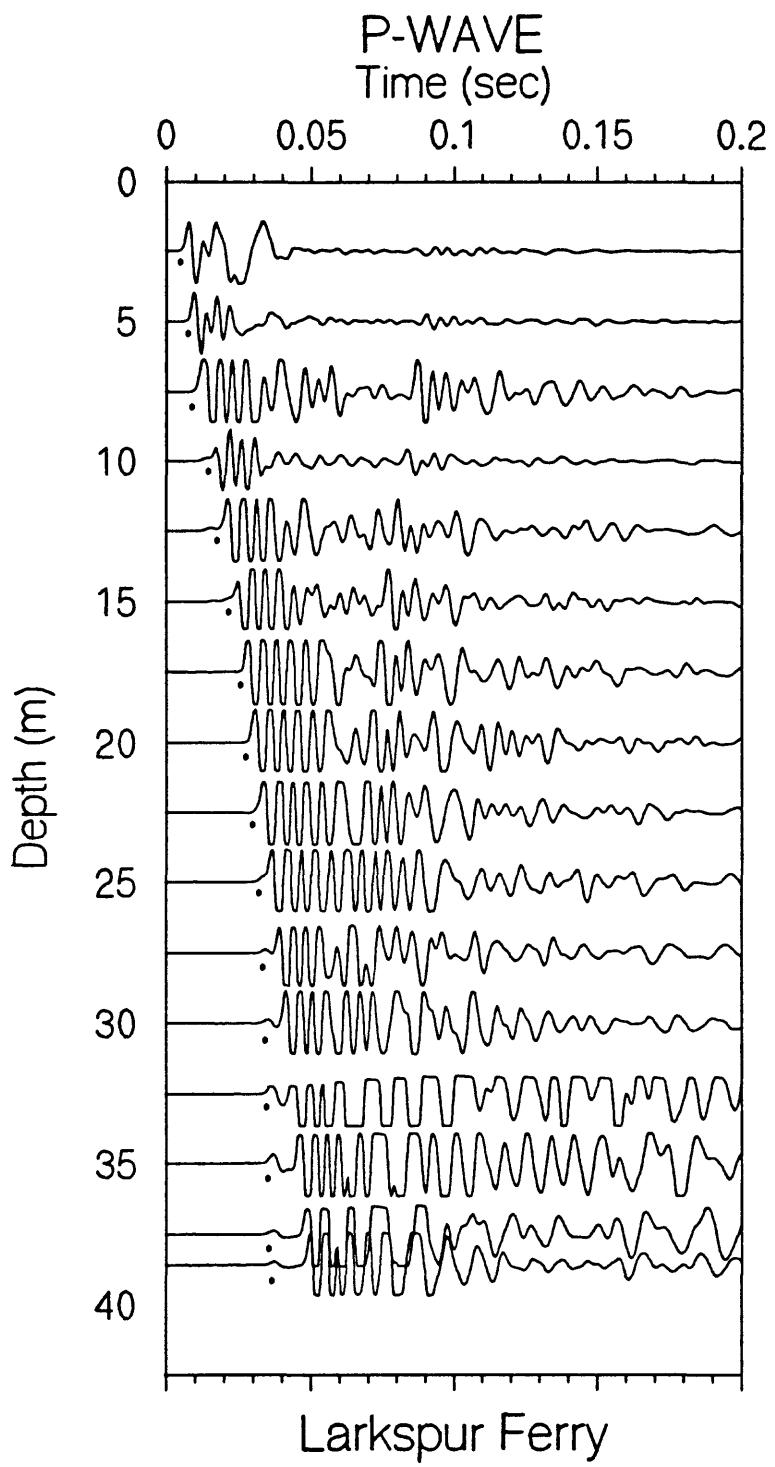


Figure 35. Vertical-component record section. P-wave arrivals are shown by the solid circles.

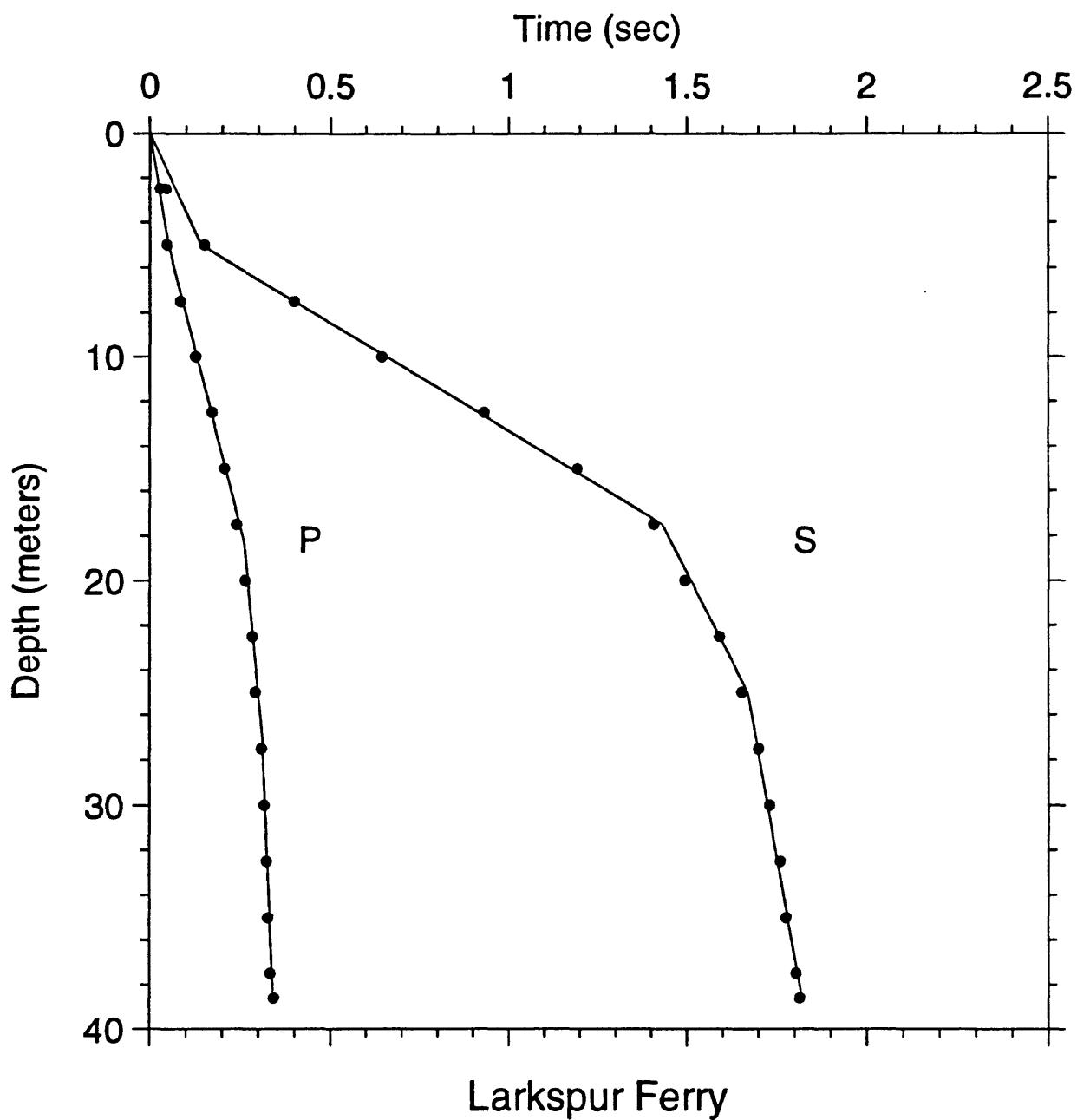


Figure 36. Time-depth graph of P-wave and S-wave picks. Line segments show the hinged-least-squares fit to the data points.

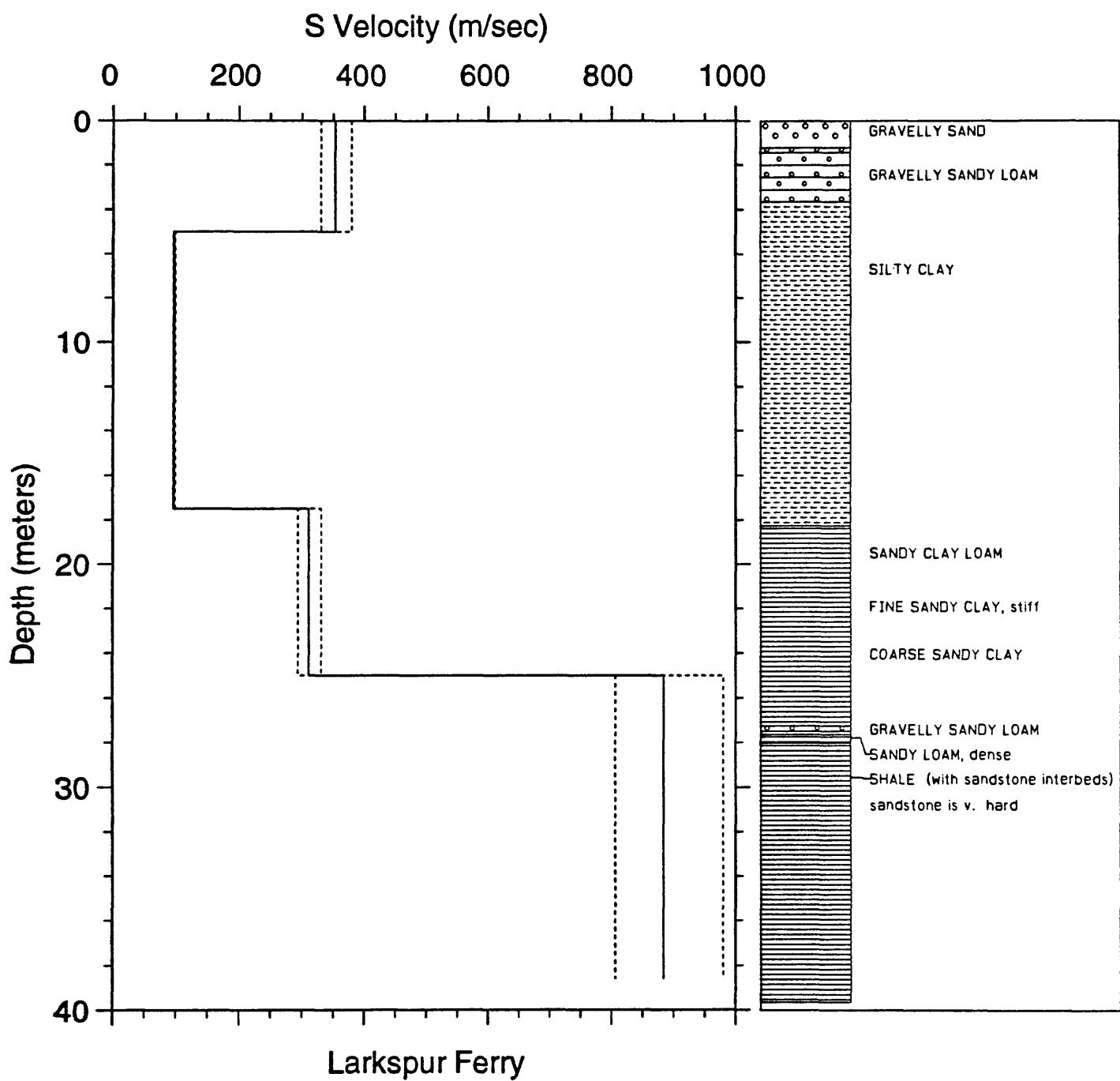


Figure 37. S-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

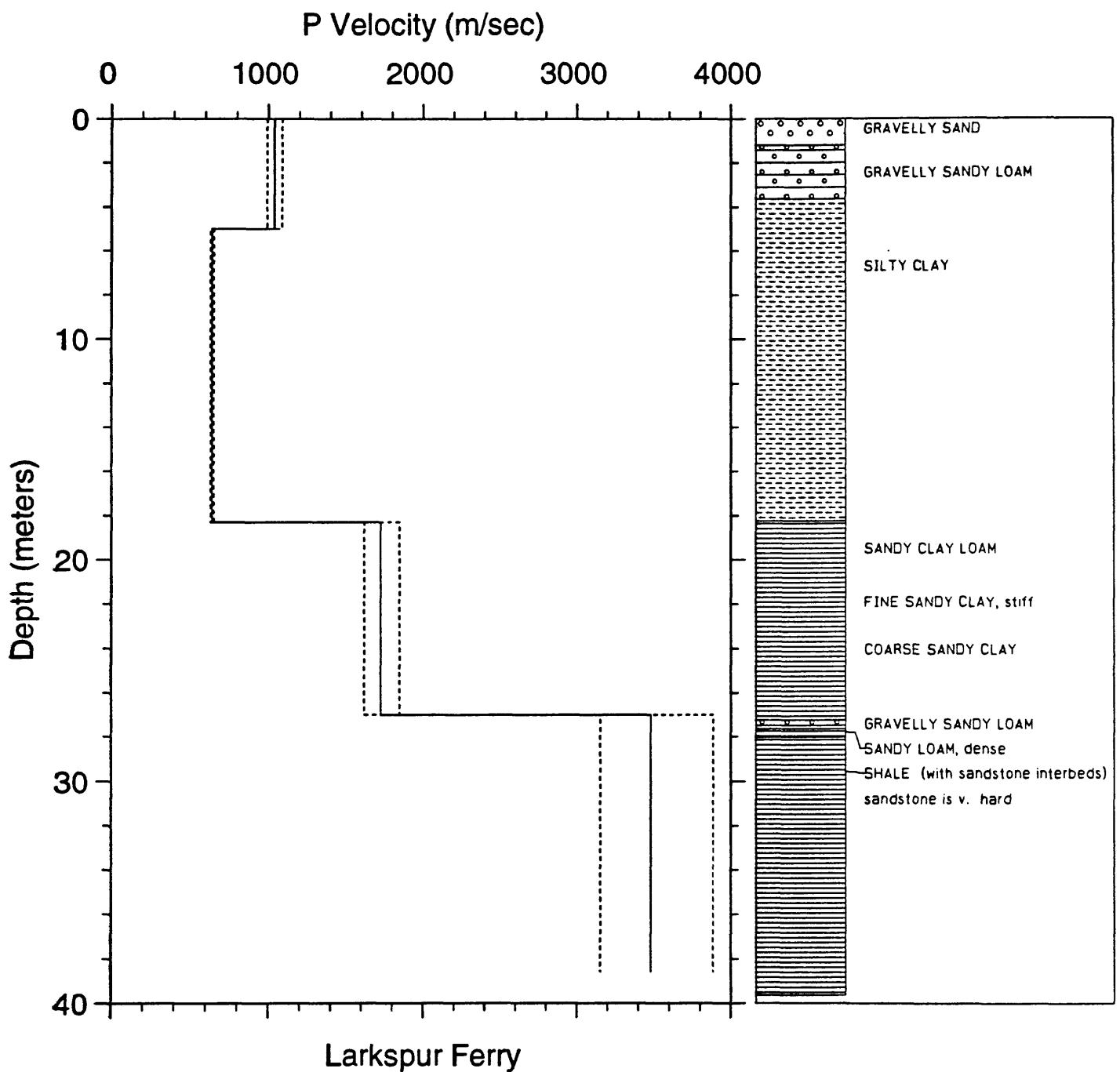


Figure 38. P-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

TABLE 7. S-wave arrival times and velocity summaries for Larkspru Ferry site.

d(m)	d(ft)	t(sec)	sig	rsdl/sig	dtb(m)	dtb(ft)	ttb(s)	v(m/s)	vl(m/s)	vu(m/s)	vt(s)	vu(ft/s)	vl(ft/s)
2.5	8.2	.0045	1	-2.6	.0	.0	.000	353	330	379	1157	1081	1245
5.0	16.4	.0149	1	-.7	5.0	16.4	.014	353	330	379	1157	1081	1245
7.5	24.6	.0399	1	-.1	17.5	57.4	.143	97	96	99	320	316	323
10.0	32.8	.0644	1	-1.1	25.0	82.0	.167	311	294	331	1022	964	1087
12.5	41.0	.0930	1	1.8	38.6	126.6	.182	884	806	980	2902	2644	3216
15.0	49.2	.1192	1	2.3									
17.5	57.4	.1407	1	-1.8									
20.0	65.6	.1493	1	-1.3									
22.5	73.8	.1590	1	-4									
25.0	82.0	.1654	1	-1.2									
27.5	90.2	.1700	1	-.6									
30.0	98.4	.1731	1	-8									
32.5	106.6	.1761	1	1.0									
35.0	114.8	.1777	1	-2									
37.5	123.0	.1804	1	-.3									
38.6	126.6	.1815	1	-.5									

Explanation:

- dsdl = least-squares residual divided by sigma
- dtb(m) = depth in meters
- dtb(ft) = depth in feet
- t(sec) = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)
- sig = sigma, standard deviation normalized to the standard deviation of best picks
- * see text for explanation of velocity limits

TABLE 8. P-wave arrival times and velocity summaries for Larkspur Ferry site.

d(m)	d(ft)	t(sec)	sig	rsdl/sig	dtb(m)	dtb(ft)	ttb(s)	v(m/s)	vl(m/s)	vu(m/s)	v(f/s)	vl(f/s)	vu(f/s)
2.5	8.2	.0028	.4	.0	1039	994	1088	3408	3260	3570			
5.0	16.4	.0046	1	.2	5.0	16.4	.005	1039	994	1088	3408	3260	3570
7.5	24.6	.0084	1	.3	18.3	60.0	.026	64.1	63.0	652	2103	2068	2139
10.0	32.8	.0126	1	.0	27.0	88.6	.031	1724	1618	1846	5657	5307	6056
12.5	41.0	.0171	1	.6	38.6	126.6	.034	3480	3152	3884	11418	10342	12742
15.0	49.2	.0206	1	.2									
17.5	57.4	.0240	1	.3									
20.0	65.6	.0263	1	.3									
22.5	73.8	.0283	1	.3									
25.0	82.0	.0292	1	.3									
27.5	90.2	.0308	1	.0									
30.0	98.4	.0316	1	.1									
32.5	106.6	.0323	1	.1									
35.0	114.8	.0327	1	.2									
37.5	123.0	.0333	1	.3									
38.6	126.6	.0343	1	.4									

Explanation:

d(m) = depth in meters

d(ft) = depth in feet

t(sec) = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)

sig = sigma, standard deviation normalized to the standard deviation of best picks

rsdl/sig = least-squares residual divided by sigma

dtb(m) = depth to bottom of layer in meters

dtb(ft) = depth to bottom of layer in feet

ttb(s) = arrival time in seconds to bottom of layer

vl(m/s) = velocity in meters per second

vl(f/s) = upper limit of velocity in meters per second *

vu(m/s) = lower limit of velocity in meters per second

v(f/s) = velocity in feet per second

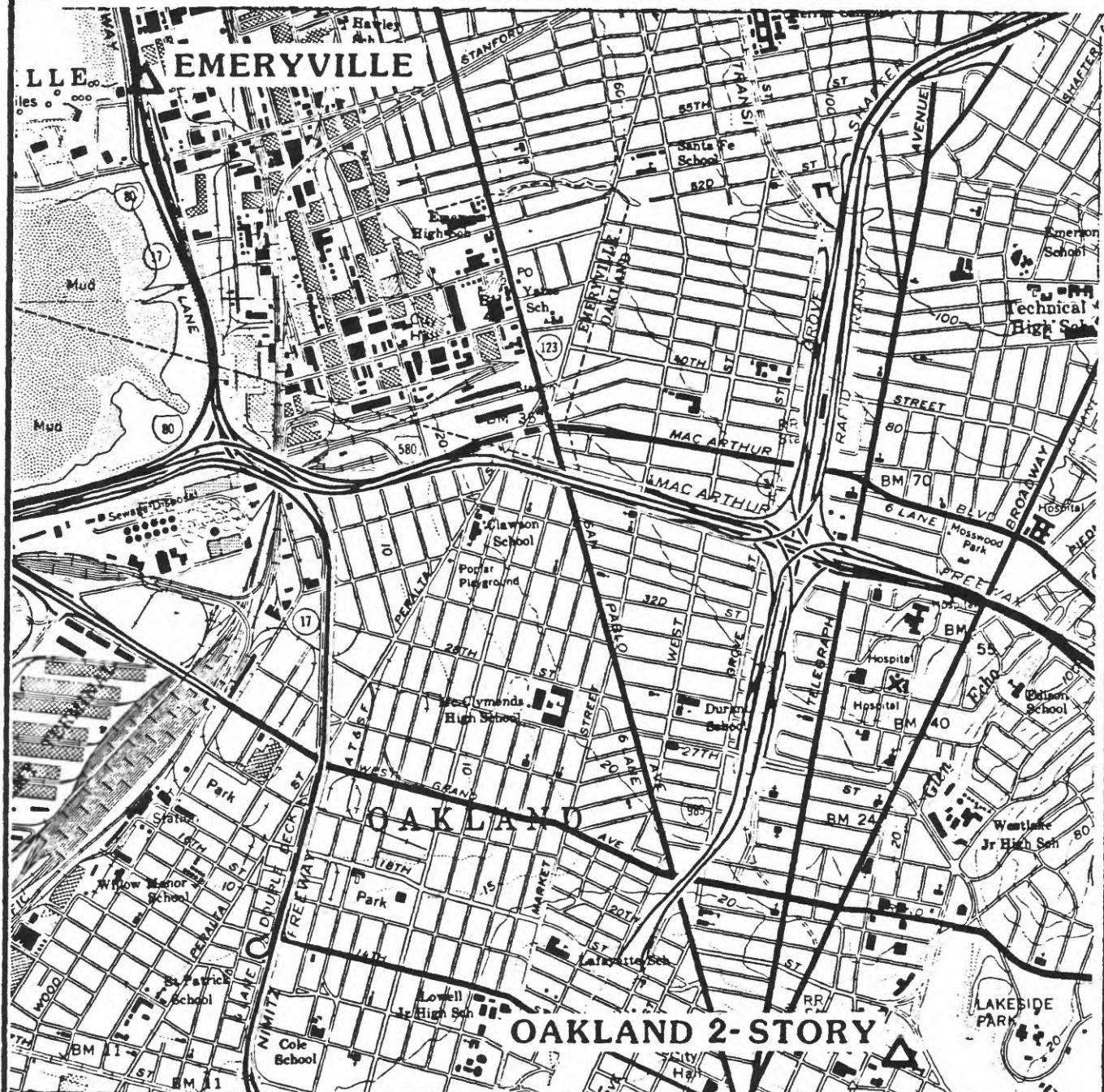
vl(f/s) = lower limit of velocity in feet per second

vu(f/s) = upper limit of velocity in feet per second

* see text for explanation of velocity limits

UNITED STATES
DEPARTMENT OF THE INTERIOR
GEOLOGICAL SURVEY

OAKLAND WEST QUADRANGLE
CALIFORNIA
7.5 MINUTE SERIES (TOPOGRAPHIC)



SCALE 1:24 000

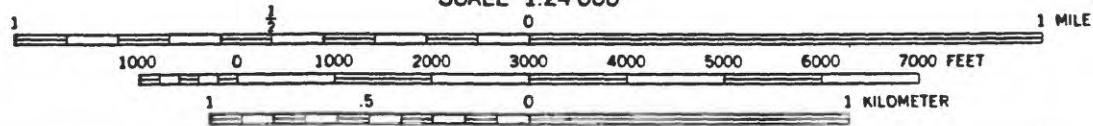


Figure 39. Site location map for Pacific Park Plaza (Emeryville) and Oakland 2-Story (not in this report). The strong-motion accelerograph is located approximately 10 meters from the borehole.

Definitions of terms used for descriptions of sedimentary deposits and bedrock materials

Rock hardness: response to hand and geologic hammer: (Ellen et al., 1972)

hard - hammer bounces off with solid sound
 firm - hammer dents with thud, pick point dents or penetrates slightly
 soft - pick points penetrates
 friable material can be crumbled into individual grains by hand.

Fracture spacing: (Ellen et al., 1972)

cm	in	fracture spacing
0-1	0-1/2	v. close
1-5	1/2-2	close
5-30	2-12	moderate
30-100	12-36	wide
>100	>36	v. wide

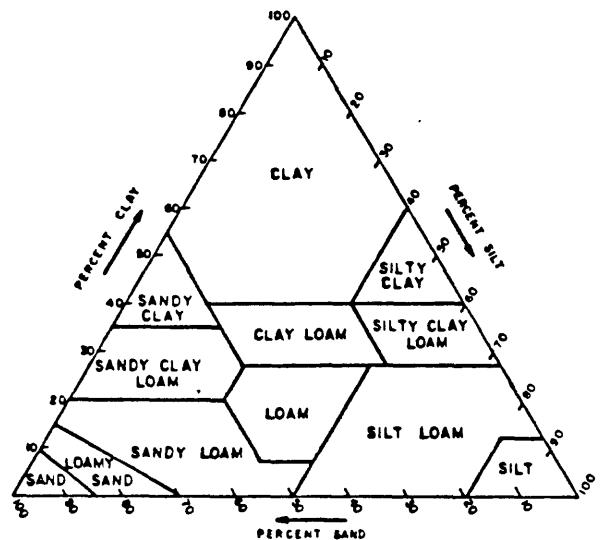
Weathering:

- Fresh: no visible signs of weathering
- Slight: no visible decomposition of minerals, slight discoloration
- Moderate: slight decomposition of minerals and disintegration of rock, deep and thorough discoloration
- Deep: extensive decomposition of minerals and complete disintegration of rock but original structure is preserved.

Relative density of sand and consistency of clay is correlated with penetration resistance: (Terzaghi and Peck, 1948)

blows/ft.	relative density	blows/ft.	consistency
0-4	v. loose	<2	v. soft
4-10	loose	2-4	soft
10-30	medium	4-8	medium
30-50	dense	8-15	stiff
>50	v. dense	15-30	v. stiff
		>30	hard

Texture: the relative proportions of clay, silt, and sand below 2mm. Proportions of larger particles are indicated by modifiers of textural class names. Determination is made in the field mainly by feeling the moist soil (Soil Survey, Staff, 1951).



Color: Standard Munsell color names are given for the dominant color of the moist soil and for prominent mottles.

Types of samples

- SP - Standard Penetration 1 + 3/8 in in ID sampler)
- S - Thin-wall push sampler
- O - Osterberg fixed-piston sampler
- P - Pitcher Barrel sampler
- CH - California Penetration (2 in ID sampler)
- DC - Diamond Core

Figure 40. Explanation of geologic logs.

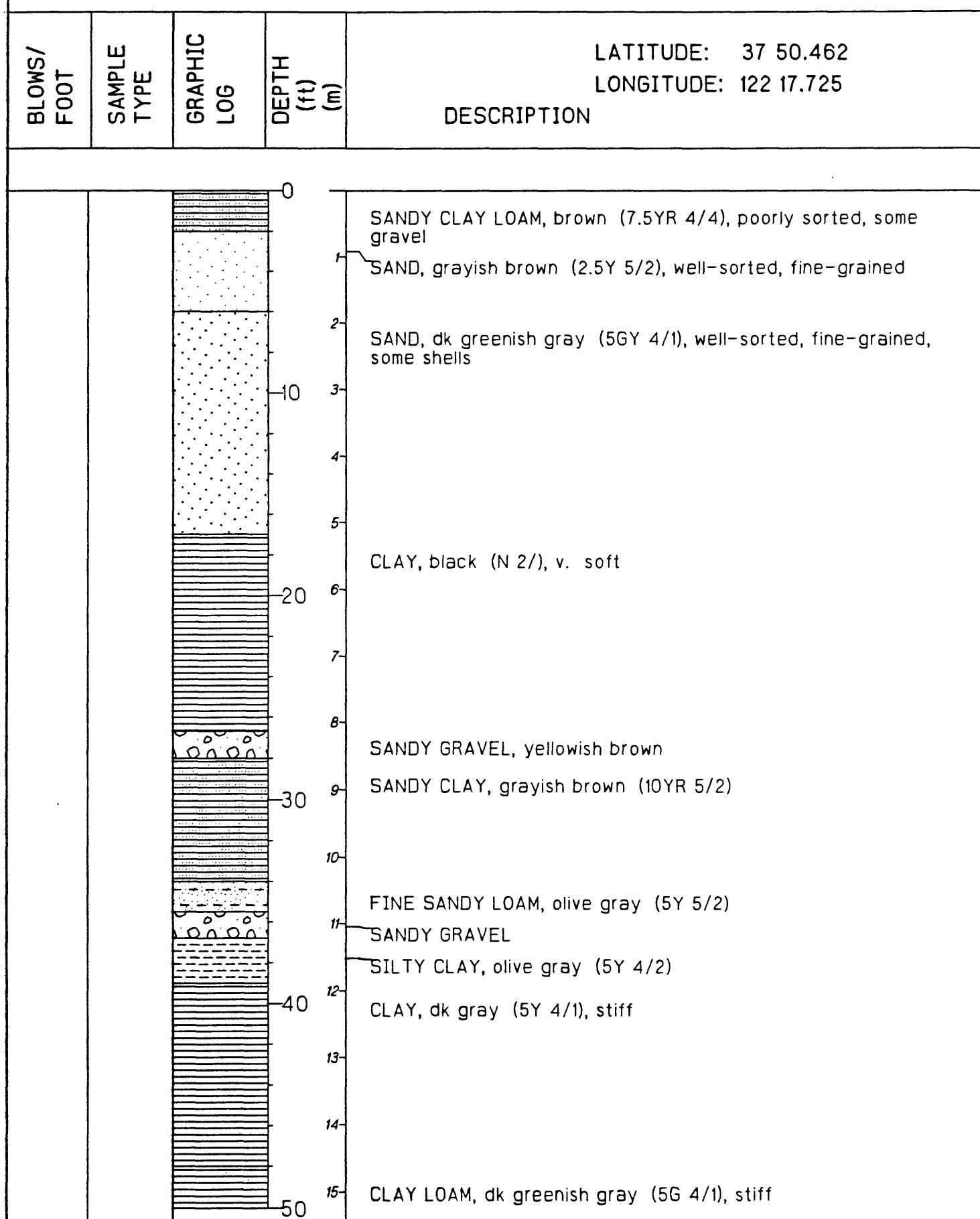


Figure 41. Geologic log of Pacific Park Plaza (Emeryville) borehole.

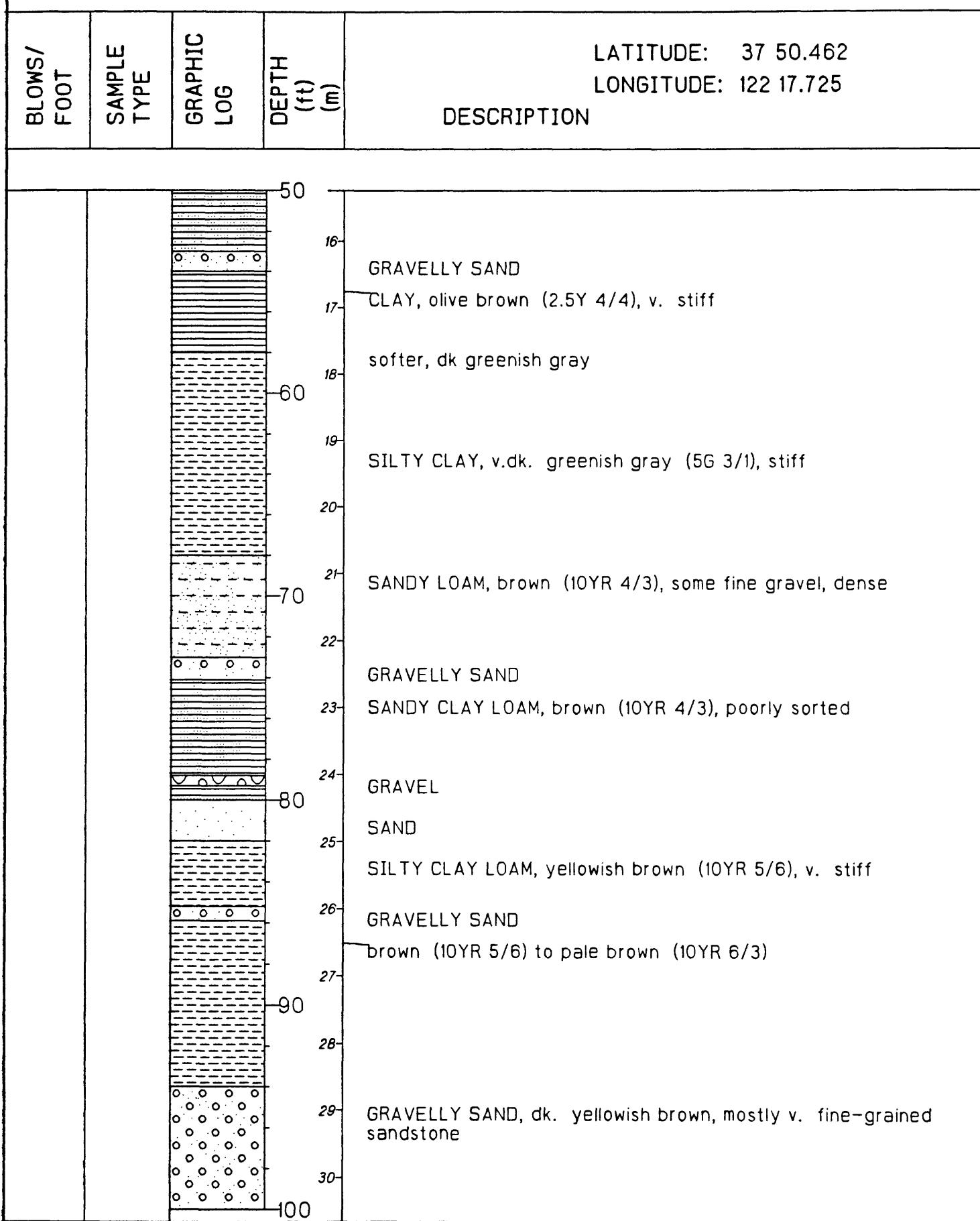


Figure 41. (Continued).

SITE: PACIFIC PARK PLAZA (EMERYVILLE)

DATE: 4/17/91

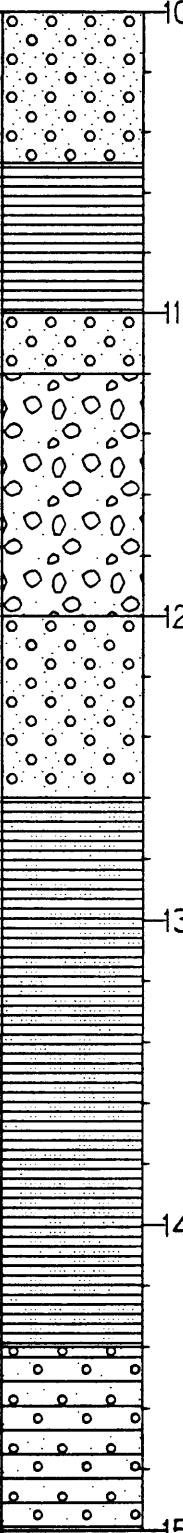
BLOWS/ FOOT	SAMPLE TYPE	GRAPHIC LOG	DEPTH (ft) (m)	DESCRIPTION
			100 31 32 33 110 34 35 36 120 37 38 39 130 40 41 42 140 43 44 45 150	LATITUDE: 37 50.462 LONGITUDE: 122 17.725
				DESCRIPTION
				CLAY LOAM, grayish brown (2.5Y 5/2), stiffer
				GRAVELLY SAND
				SANDY GRAVEL
				GRAVELLY SAND
				SANDY CLAY LOAM, light olive brown (2.5Y 5/4), v. stiff
				lt. brownish gray (2.5Y 6/2), some gravel
				lt. olive brown (2.5Y 5/4)
				GRAVELLY SANDY LOAM

Figure 41. (Continued).

SITE: PACIFIC PARK PLAZA (EMERYVILLE)

DATE: 4/17/91

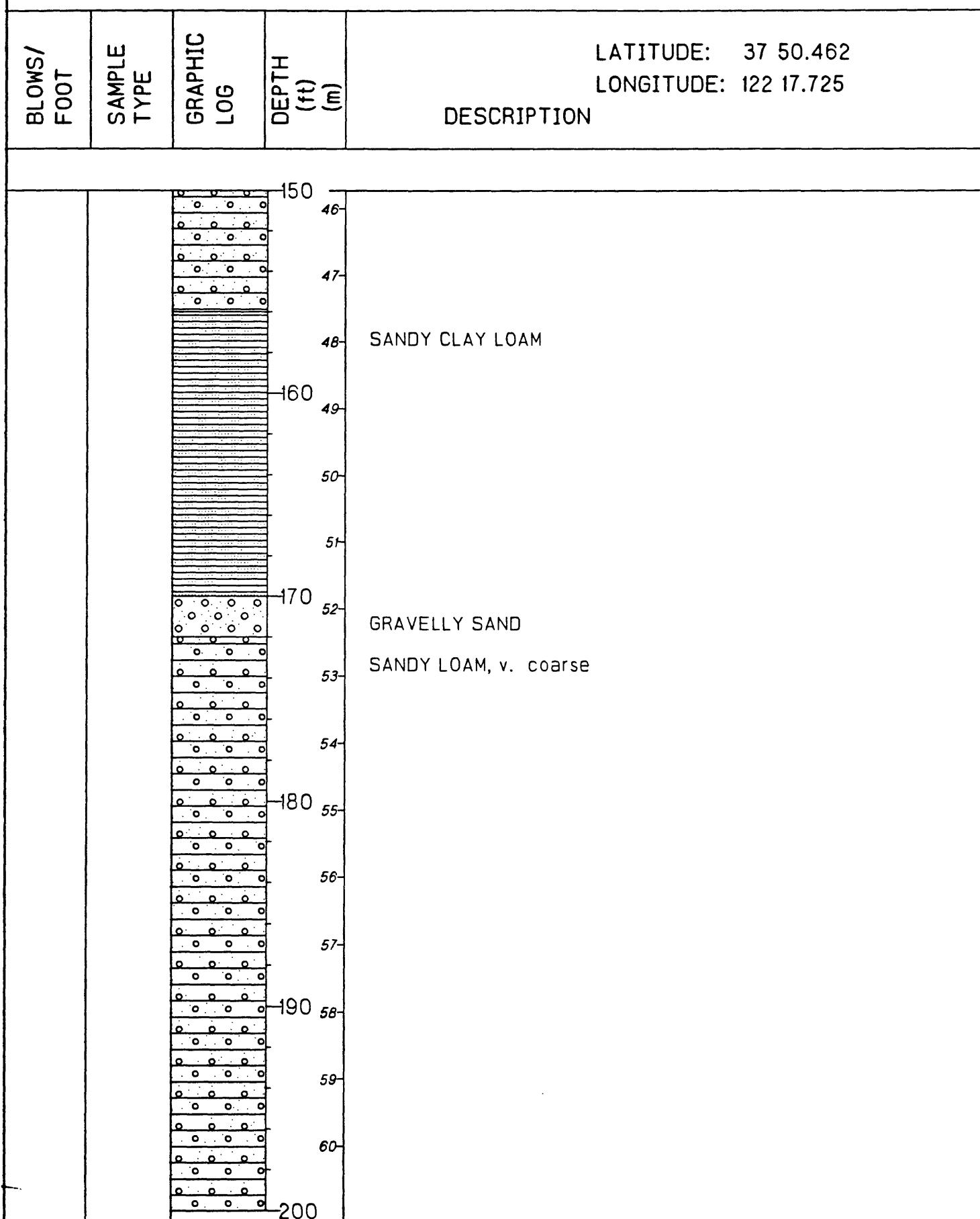


Figure 41. (Continued).

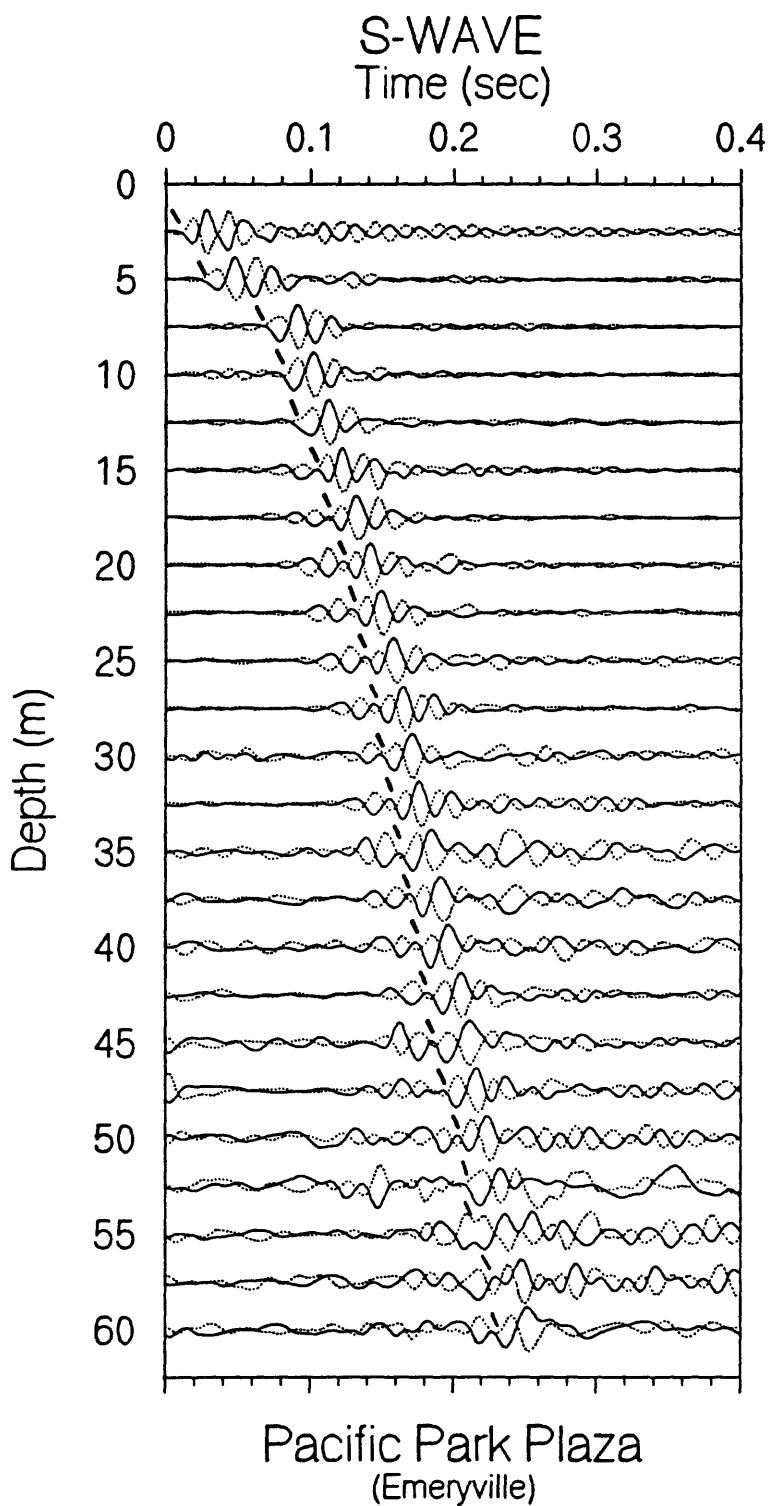


Figure 42. Horizontal-component record section (from horizontal impacts in opposite directions) superimposed for identification of S-wave onset. Approximate S-wave picks are indicated by the accent marks.

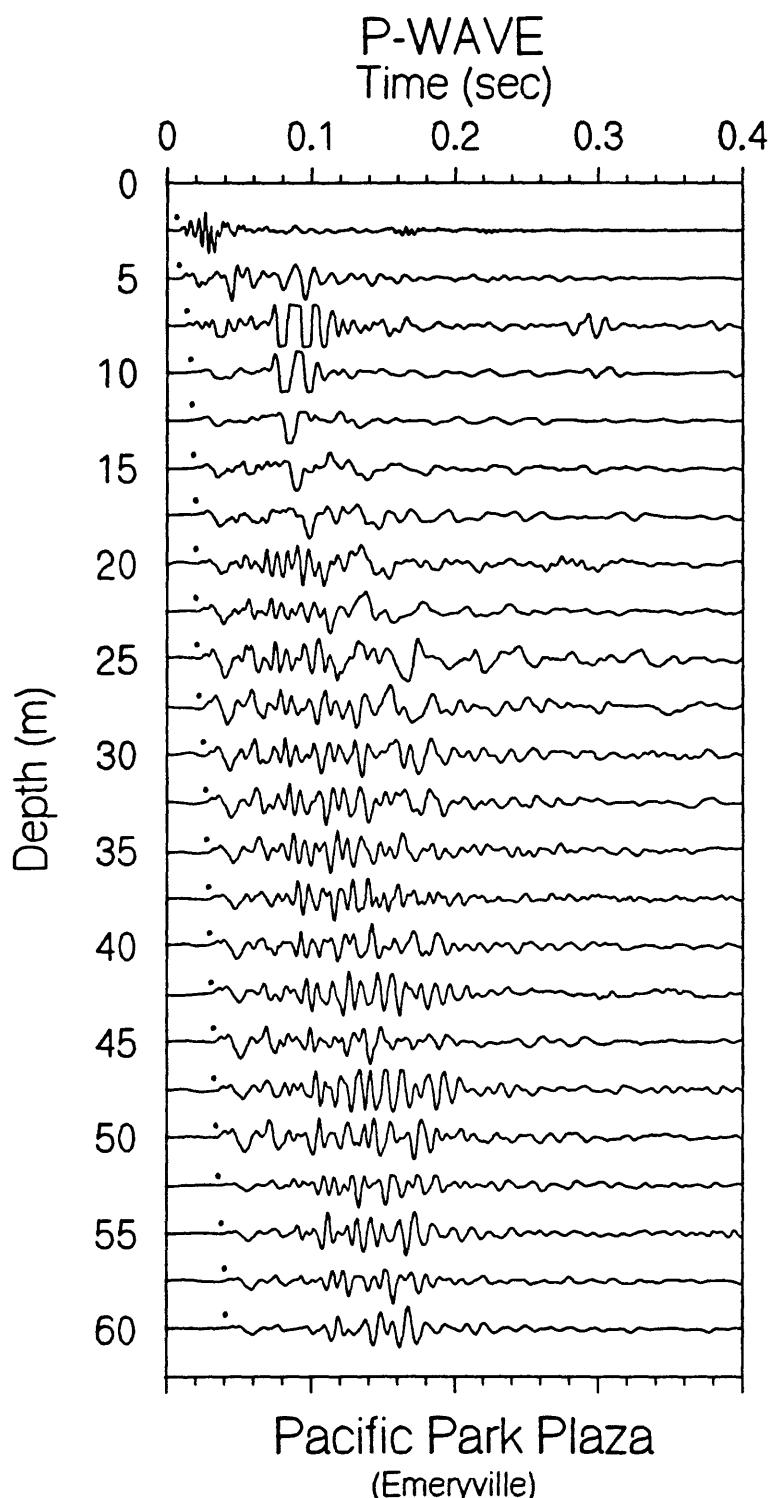


Figure 43. Vertical-component record section. P-wave arrivals are shown by the solid circles.

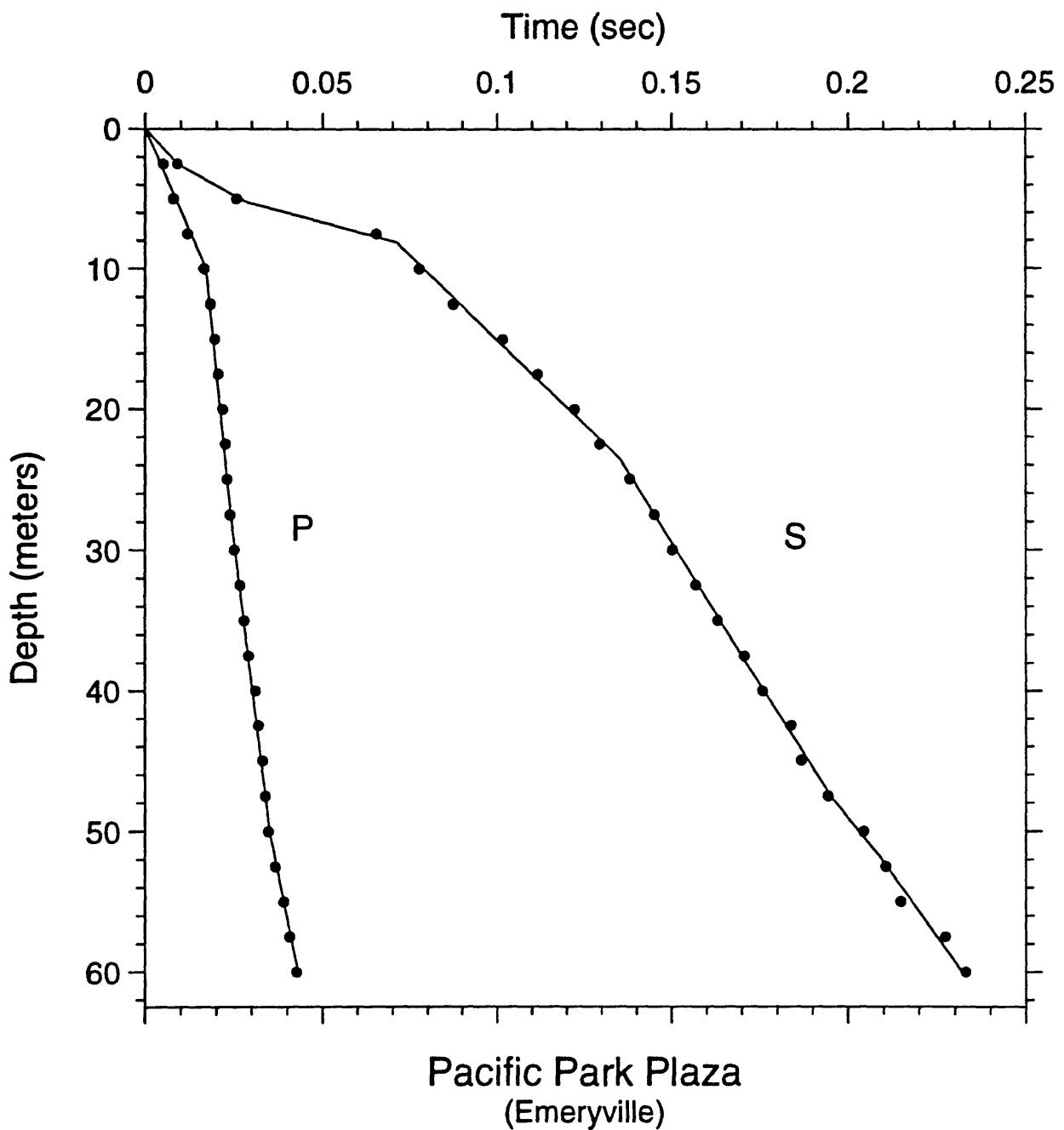


Figure 44. Time-depth graph of P-wave and S-wave picks. Line segments show the hinged-least-squares fit to the data points.

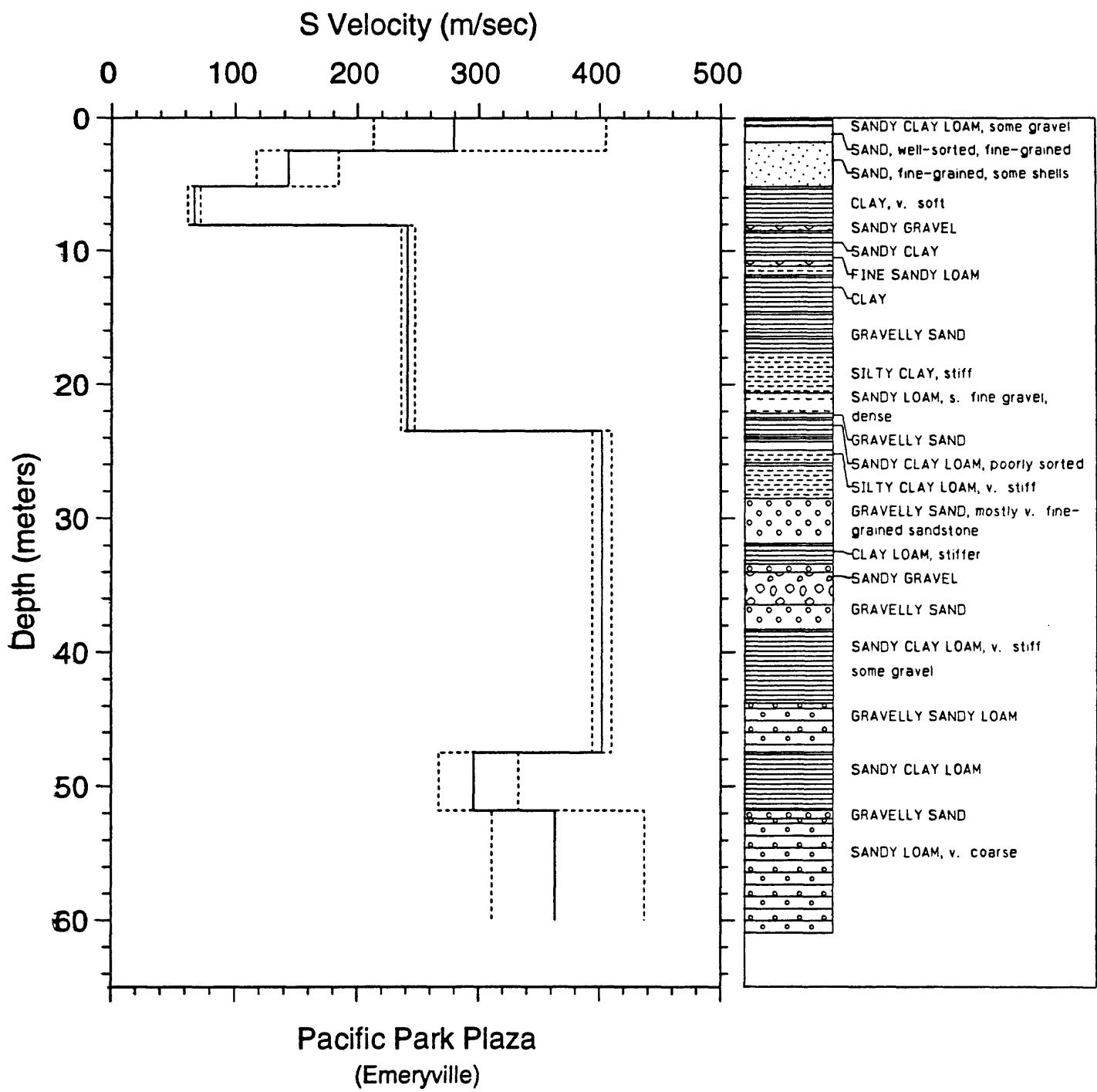


Figure 45. S-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

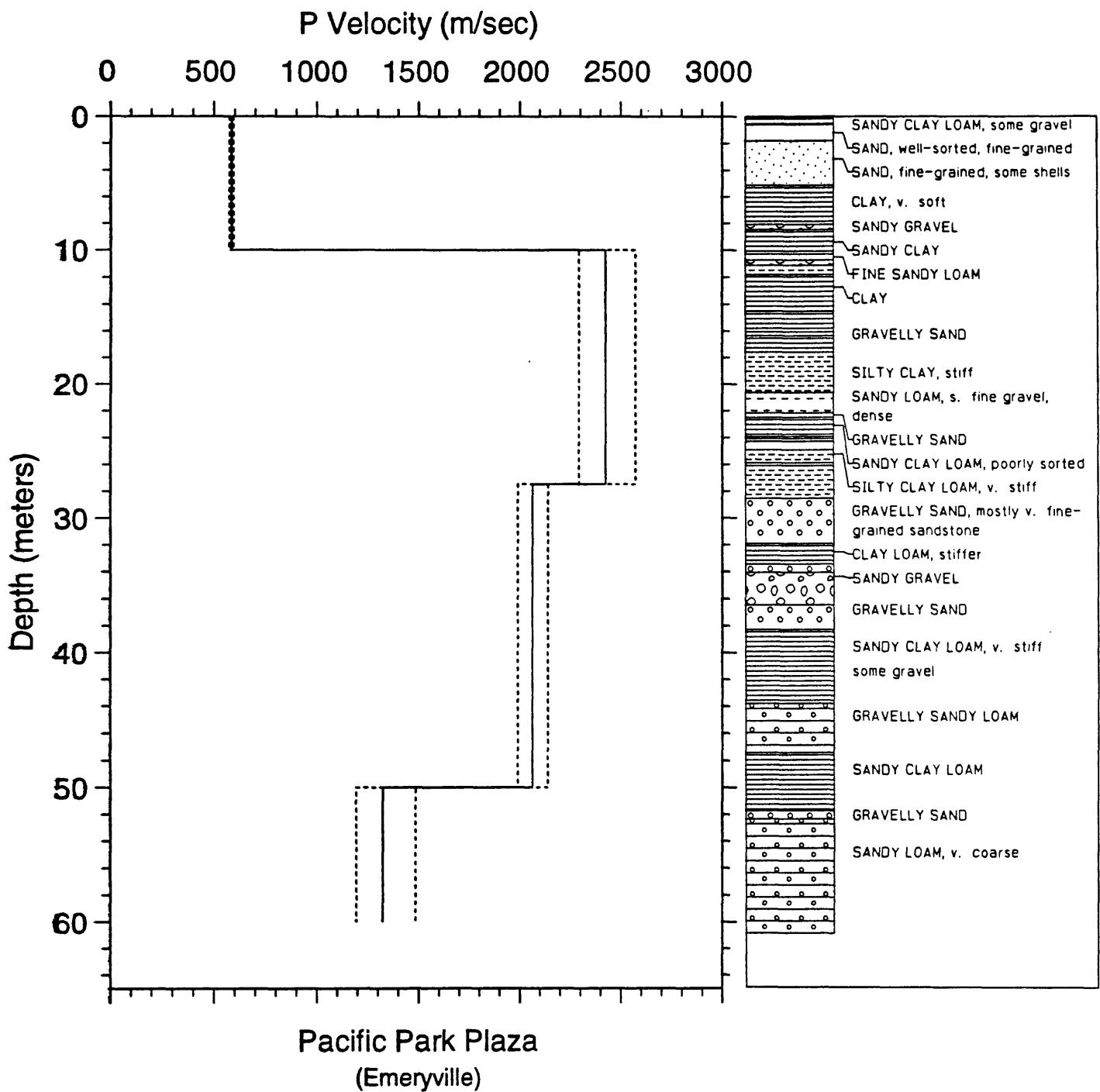


Figure 46. P-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

TABLE 9. S-wave arrival times and velocity summaries for Pacific Park Plaza site.

d(ft)	t(sec)	sig	rsdl/sig	dtb(m)	dtb(ft)	ttb(s)	v(m/s)	vl(m/s)	vu(m/s)	v(ft/s)	vl(ft/s)	vu(ft/s)
2.5	8.2	.0090	2	.0	.0	.000	279	213	405	917	699	1330
5.0	16.4	.0257	2	.4	2.5	.2	279	213	405	917	699	1330
7.5	24.6	.0653	2	1.6	5.2	17.1	.028	143	117	184	470	384
10.0	32.8	.0776	1	-1.3	8.1	26.6	.071	67	62	72	220	205
12.5	41.0	.0873	1	-2.0	23.5	77.1	.135	241	236	247	791	773
15.0	49.2	.1016	1	1.9	47.5	155.8	.195	402	394	410	1319	1293
17.5	57.4	.1116	1	1.5	51.8	169.9	.209	296	267	333	972	876
20.0	65.6	.1222	1	1.8	60.0	196.9	.232	363	311	437	1191	1092
22.5	73.8	.1294	1	-1.4								
25.0	82.0	.1380	1	-7								
27.5	90.2	.1451	1	-2								
30.0	98.4	.1502	1	-9								
32.5	106.6	.1569	1	-4								
35.0	114.8	.1632	1	-4								
37.5	123.0	.1707	1	-9								
40.0	131.2	.1759	1	-1								
42.5	139.4	.1839	1	1.7								
45.0	147.6	.1868	1	-1.6								
47.5	155.8	.1944	1	-3								
50.0	164.0	.2044	1	1.3								
52.5	172.2	.2107	1	-4								
55.0	180.4	.2149	2	-1.5								
57.5	188.6	.2273	3	.4								
60.0	196.9	.2331	3									

Explanation:

d(m) = depth in meters

d(ft) = depth in feet

t(sec) = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)

sig = sigma, standard deviation normalized to the standard deviation of best picks

rsdl/sig = least-squares residual divided by sigma

dtb(m) = depth to bottom of layer in meters

dtb(ft) = depth to bottom of layer in feet

ttb(s) = arrival time in seconds to bottom of layer

v(m/s) = velocity in meters per second

vl(m/s) = lower limit of velocity in meters per second

vu(m/s) = upper limit of velocity in meters per second

v(ft/s) = velocity in feet per second

vl(ft/s) = lower limit of velocity in feet per second

vu(ft/s) = upper limit of velocity in feet per second

* see text for explanation of velocity limits

TABLE 10. P-wave arrival times and velocity summaries for Pacific Park Plaza site.

d(m)	d(ft)	t(sec)	sig	rsdl/sig	dtb(m)	dtb(ft)	ttb(s)	v(m/s)	v(ft/s)	vu(m/s)	vu(ft/s)	vl(m/s)	vl(ft/s)	vu(ft/s)	vl(ft/s)
2.5	8.2	.0051	1	.8	10.0	32.8	.000	584	574	594	594	1916	1884	1949	1949
5.0	16.4	.0079	1	.7	10.0	32.8	.017	584	574	594	594	1916	1884	1949	1949
7.5	24.6	.0119	1	.9	27.5	90.2	.024	2424	2292	2572	2572	7954	7521	8439	8439
10.0	32.8	.0165	4	.2	50.0	164.0	.035	2062	1991	2140	2140	6767	6531	7020	7020
12.5	41.0	.0183	1	.1	60.0	196.9	.043	1324	1194	1485	1485	4343	3917	4873	4873
15.0	49.2	.0195	1	.4											
17.5	57.4	.0205	1	.3											
20.0	65.6	.0218	1	.6											
22.5	73.8	.0226	1	.3											
25.0	82.0	.0230	1	.3											
27.5	90.2	.0239	1	.4											
30.0	98.4	.0251	1	.5											
32.5	106.6	.0267	1	.1											
35.0	114.8	.0279	1	.1											
37.5	123.0	.0291	1	.1											
40.0	131.2	.0311	1	.7											
42.5	139.4	.0319	1	.3											
45.0	147.6	.0331	1	.3											
47.5	155.8	.0339	1	.1											
50.0	164.0	.0347	1	.5											
52.5	172.2	.0367	1	.1											
55.0	180.4	.0391	1	.1											
57.5	188.6	.0407	3	.1											
60.0	196.9	.0427	3	.0											

Explanation:

d(m) = depth in meters

d(ft) = depth in feet

t(sec) = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)

sig = sigma, standard deviation normalized to the standard deviation of best picks

rsdl/sig = least-squares residual divided by sigma

dtb(m) = depth to bottom of layer in meters

dtb(ft) = depth to bottom of layer in feet

ttb(s) = arrival time in seconds to bottom of layer

v(m/s) = velocity in meters per second

vl(m/s) = lower limit of velocity in meters per second *

vu(m/s) = upper limit of velocity in meters per second

v(f/s) = velocity in feet per second

vl(f/s) = lower limit of velocity in feet per second

vu(f/s) = upper limit of velocity in feet per second

* see text for explanation of velocity limits

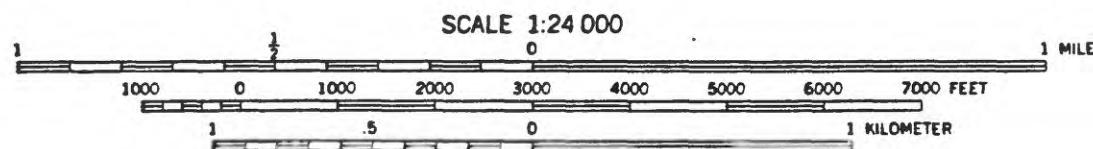


Figure 47. Location map for Stanford Linear Accelerator (SLAC) borehole. The borehole is located approximately 300 meters from the strong-motion recorder.

Definitions of terms used for descriptions of sedimentary deposits and bedrock materials

Rock hardness: response to hand and geologic hammer: (Ellen et al., 1972)

hard - hammer bounces off with solid sound
 firm - hammer dents with thud, pick point dents or penetrates slightly
 soft - pick points penetrates
 friable material can be crumbled into individual grains by hand.

Fracture spacing: (Ellen et al., 1972)

cm	in	fracture spacing
0-1	0-1/2	v. close
1-5	1/2-2	close
5-30	2-12	moderate
30-100	12-36	wide
>100	>36	v. wide

Weathering:

Fresh: no visible signs of weathering

Slight: no visible decomposition of minerals, slight discoloration

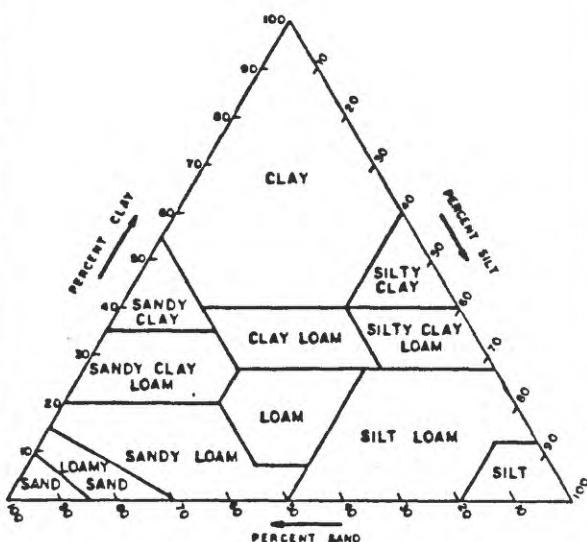
Moderate: slight decomposition of minerals and disintegration of rock, deep and thorough discoloration

Deep: extensive decomposition of minerals and complete disintegration of rock but original structure is preserved.

Relative density of sand and consistency of clay is correlated with penetration resistance: (Terzaghi and Peck, 1948)

blows/ft.	relative density	blows/ft.	consistency
0-4	v. loose	<2	v. soft
4-10	loose	2-4	soft
10-30	medium	4-8	medium
30-50	dense	8-15	stiff
>50	v. dense	15-30	v. stiff
		>30	hard

Texture: the relative proportions of clay, silt, and sand below 2mm. Proportions of larger particles are indicated by modifiers of textural class names. Determination is made in the field mainly by feeling the moist soil (Soil Survey, Staff, 1951).



Color: Standard Munsell color names are given for the dominant color of the moist soil and for prominent mottles.

Types of samples

SP - Standard Penetration 1 + 3/8 in in ID sampler)

S - Thin-wall push sampler

O - Osterberg fixed-piston sampler

P - Pitcher Barrel sampler

CH - California Penetration (2 in ID sampler)

DC - Diamond Core

Figure 48. Explanation of geologic log.

SITE: SLAC

DATE: 4/9/91

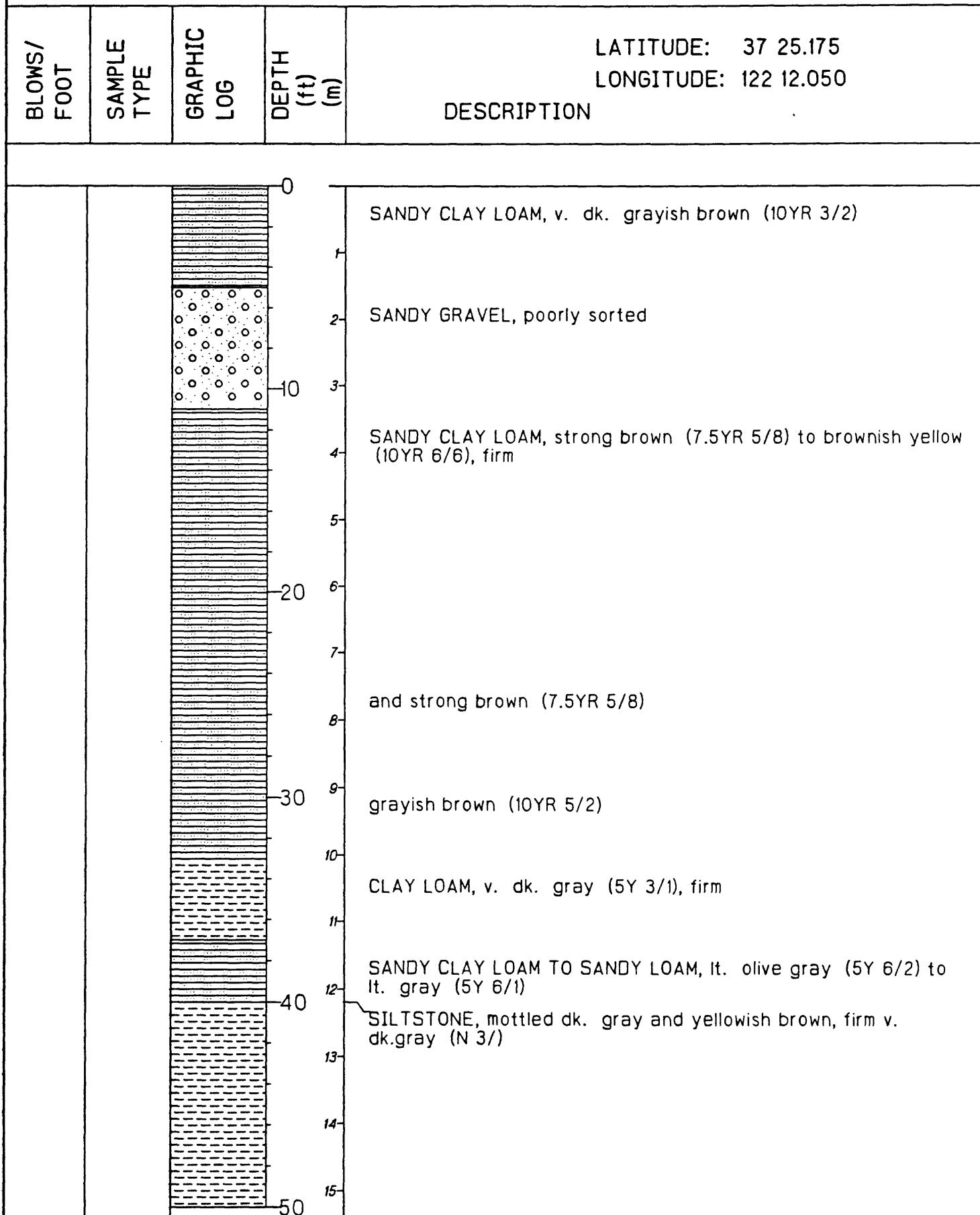


Figure 49. Geologic log of SLAC borehole.

SITE: SLAC

DATE: 4/9/91

BLOWS/ FOOT	SAMPLE TYPE	GRAPHIC LOG	DEPTH (ft) (m)	DESCRIPTION
			50	
			16	
			17	
			18	
			60	
			19	
			20	
			21	
			70	
			22	
			23	SANDSTONE, lt. gray, harder
			24	SILTSTONE, v. dk. gray
			80	SANDSTONE, v. firm
			25	SILTSTONE, firm
			26	SANDSTONE
			27	SILTSTONE
			90	
			28	
			29	SANDSTONE, dk. gray, v. fine-grained hard
			30	
			100	

The stratigraphic log shows the following sequence of rock units from top to bottom:

- Top layer (dashed pattern): SANDSTONE, lt. gray, harder
- Second layer (solid gray): SILTSTONE, v. dk. gray
- Third layer (dashed pattern): SANDSTONE, v. firm
- Fourth layer (solid gray): SILTSTONE, firm
- Fifth layer (dashed pattern): SANDSTONE
- Sixth layer (solid gray): SILTSTONE
- Bottom layer (dashed pattern): SANDSTONE, dk. gray, v. fine-grained hard

Figure 49. (Continued).

SITE: SLAC

DATE: 4/9/91

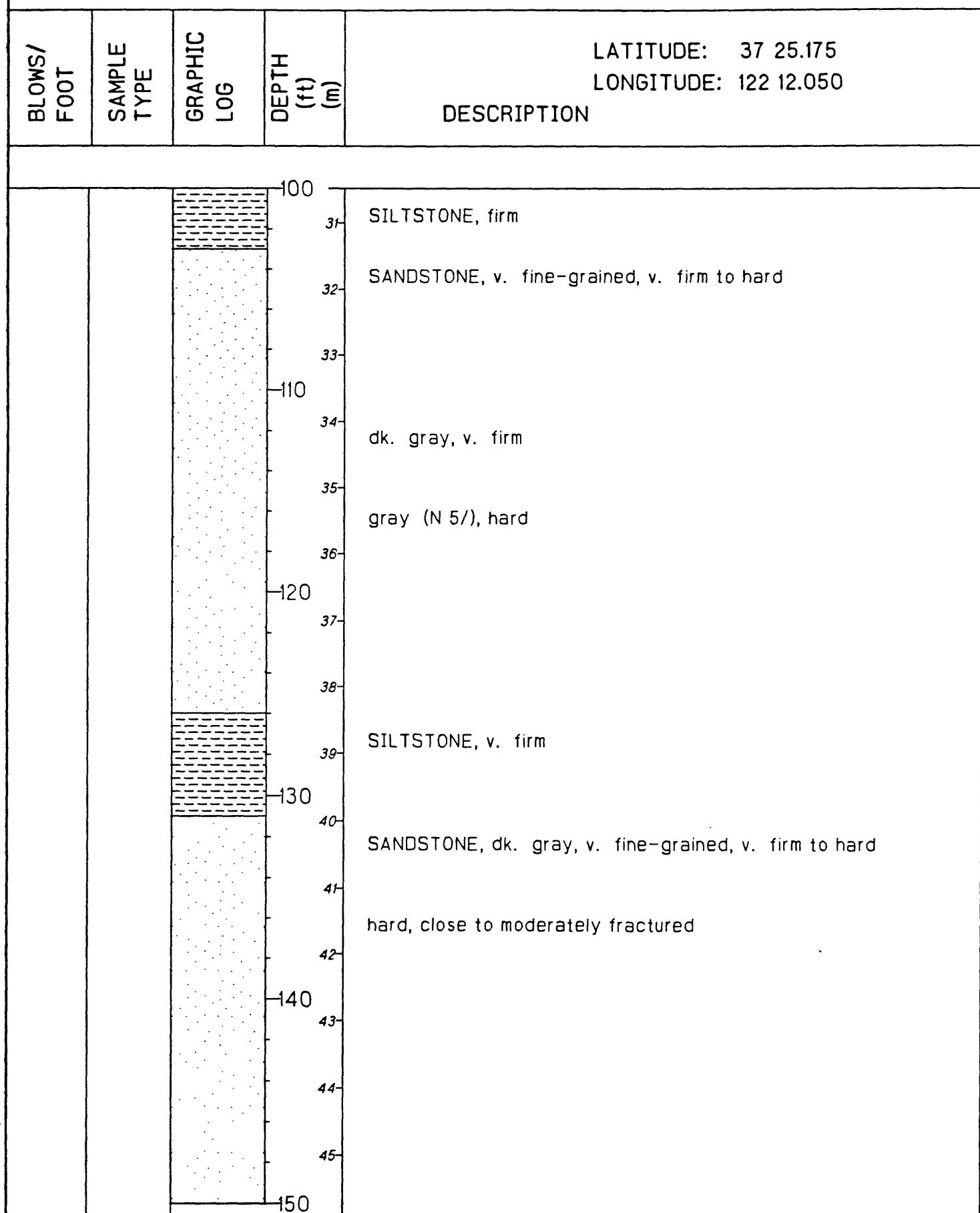


Figure 49. (Continued).

SITE: SLAC

DATE: 4/9/91

BLOWS/ FOOT	SAMPLE TYPE	GRAPHIC LOG	DEPTH (ft) (m)	LATITUDE: 37 25.175 LONGITUDE: 122 12.050 DESCRIPTION
				<p>150 46 47 48 SILTSTONE, v. firm 160 49 50 51 52 53 54 55 56 57 58 59 60 SANDSTONE, fine-grained, hard 200</p>

Figure 49. (Continued).

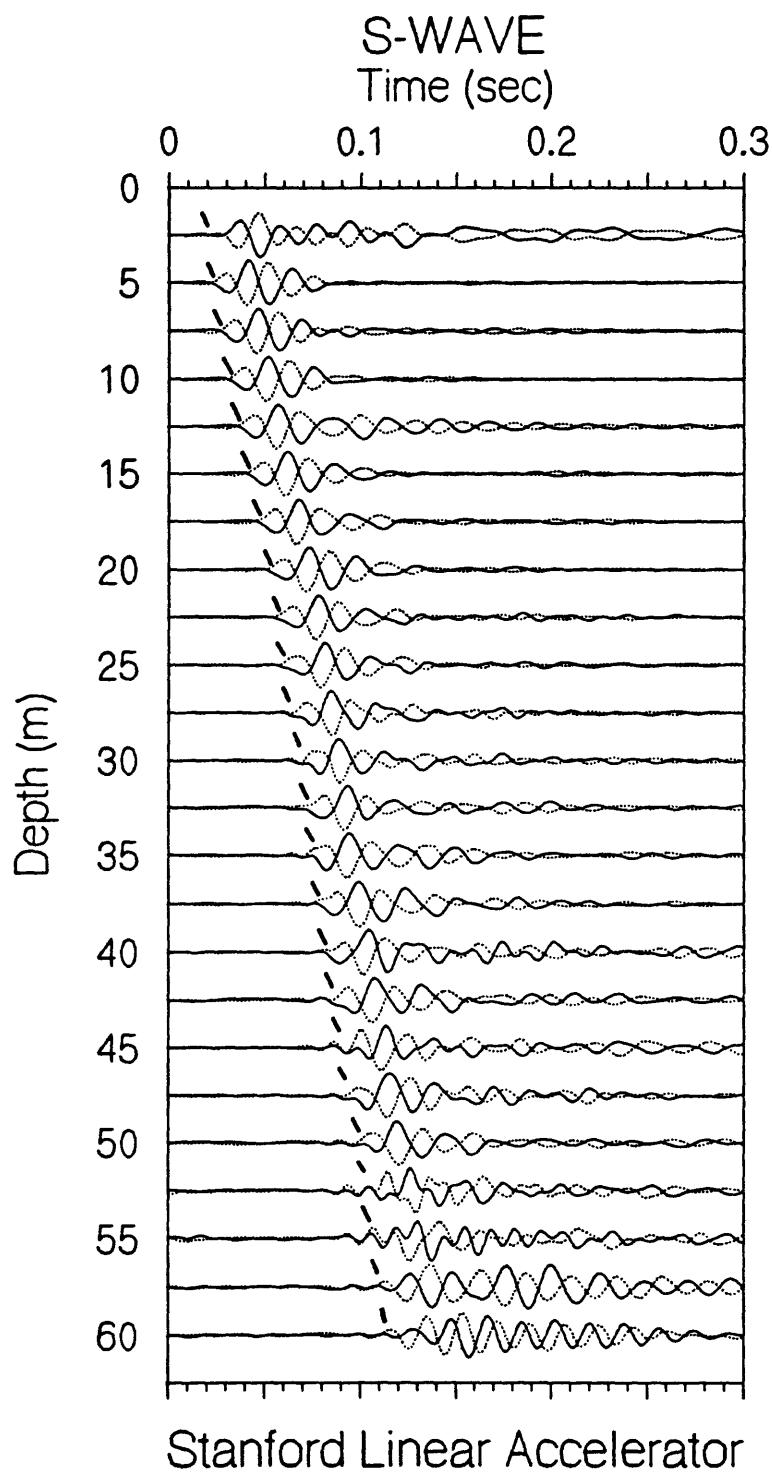


Figure 50. Horizontal-component record section (from horizontal impacts in opposite directions) superimposed for identification of S-wave onset. Approximate S-wave picks are indicated by the accent marks.

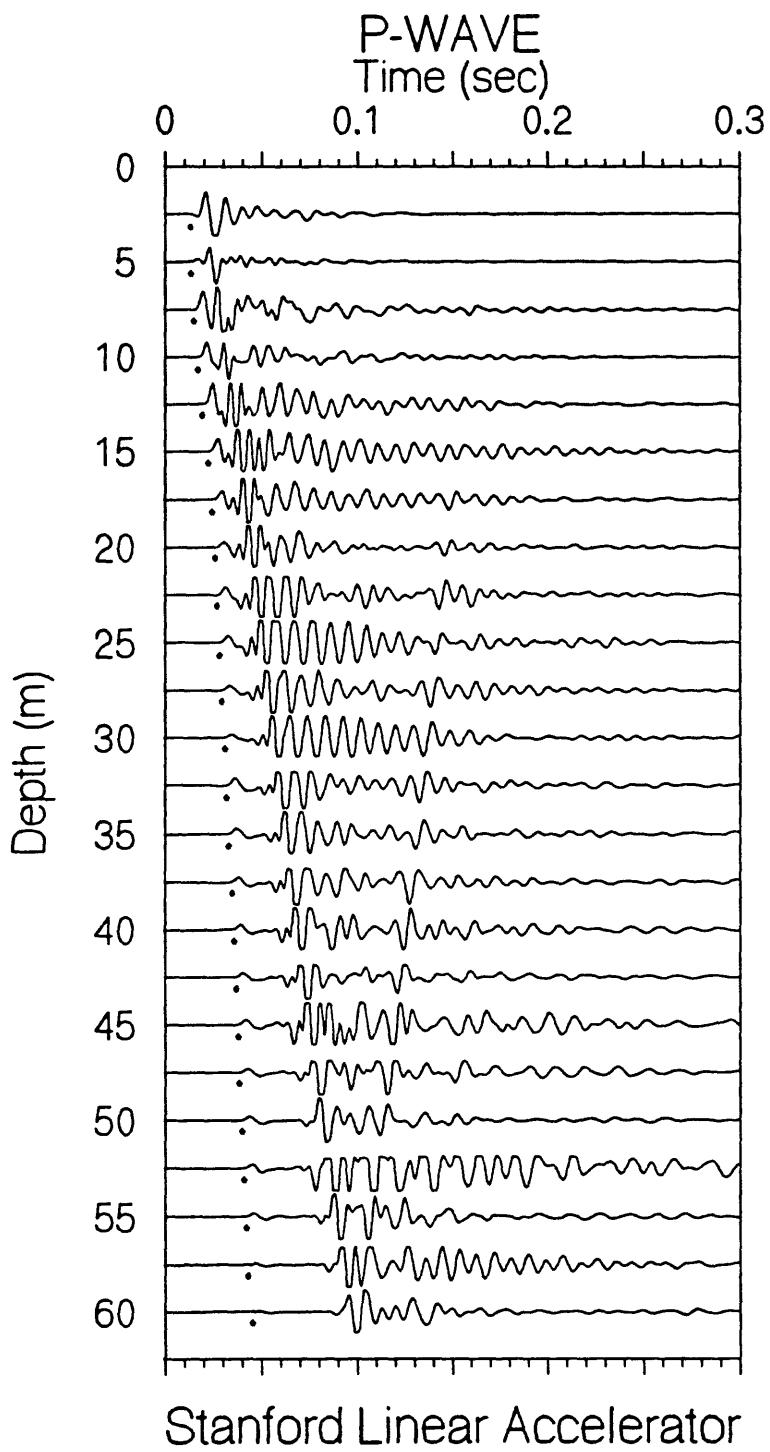


Figure 51. Vertical-component record section. P-wave arrivals are shown by the solid circles.

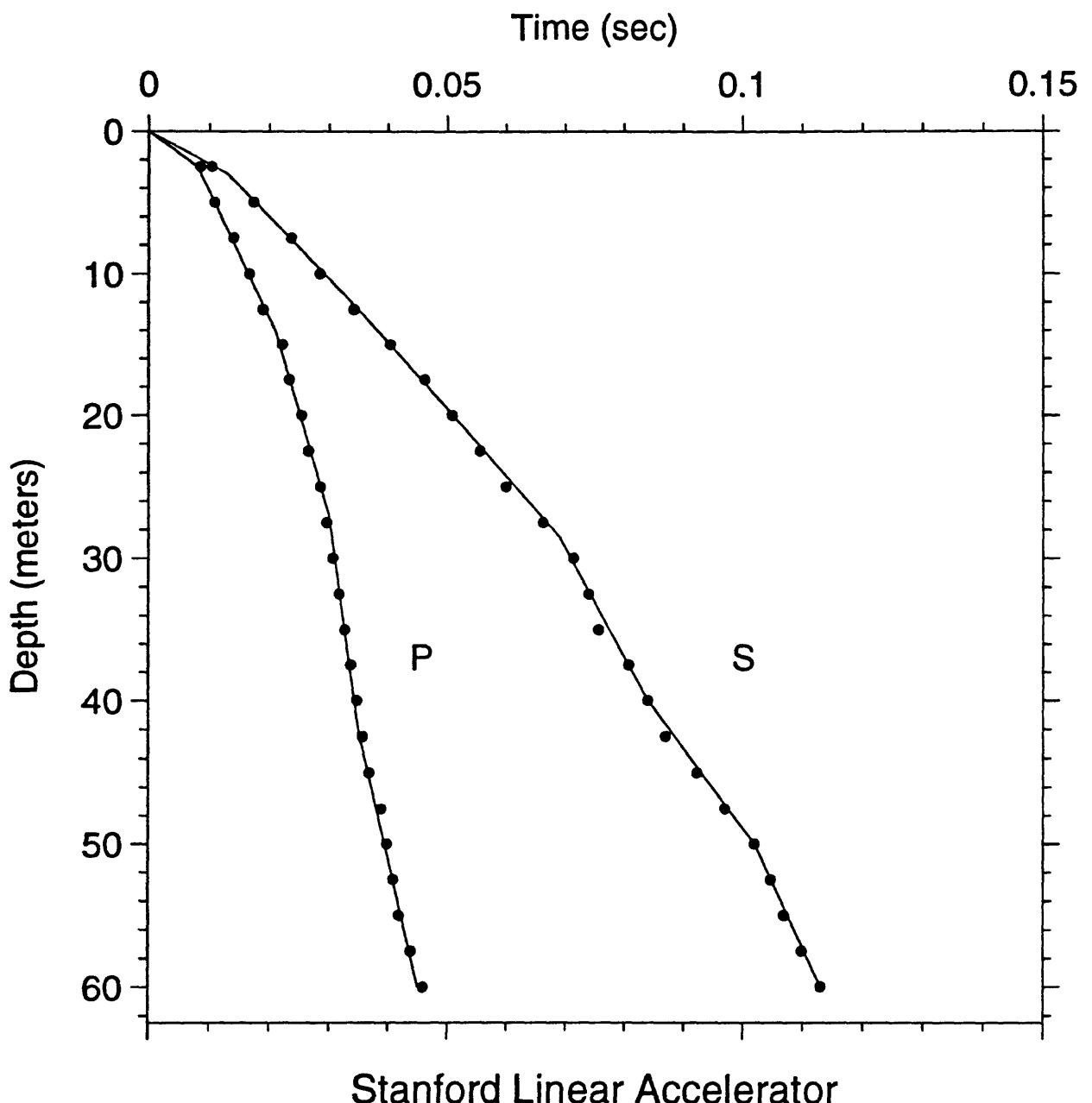


Figure 52. Time-depth graph of P-wave and S-wave picks. Line segments show the hinged-least-squares fit to the data points.

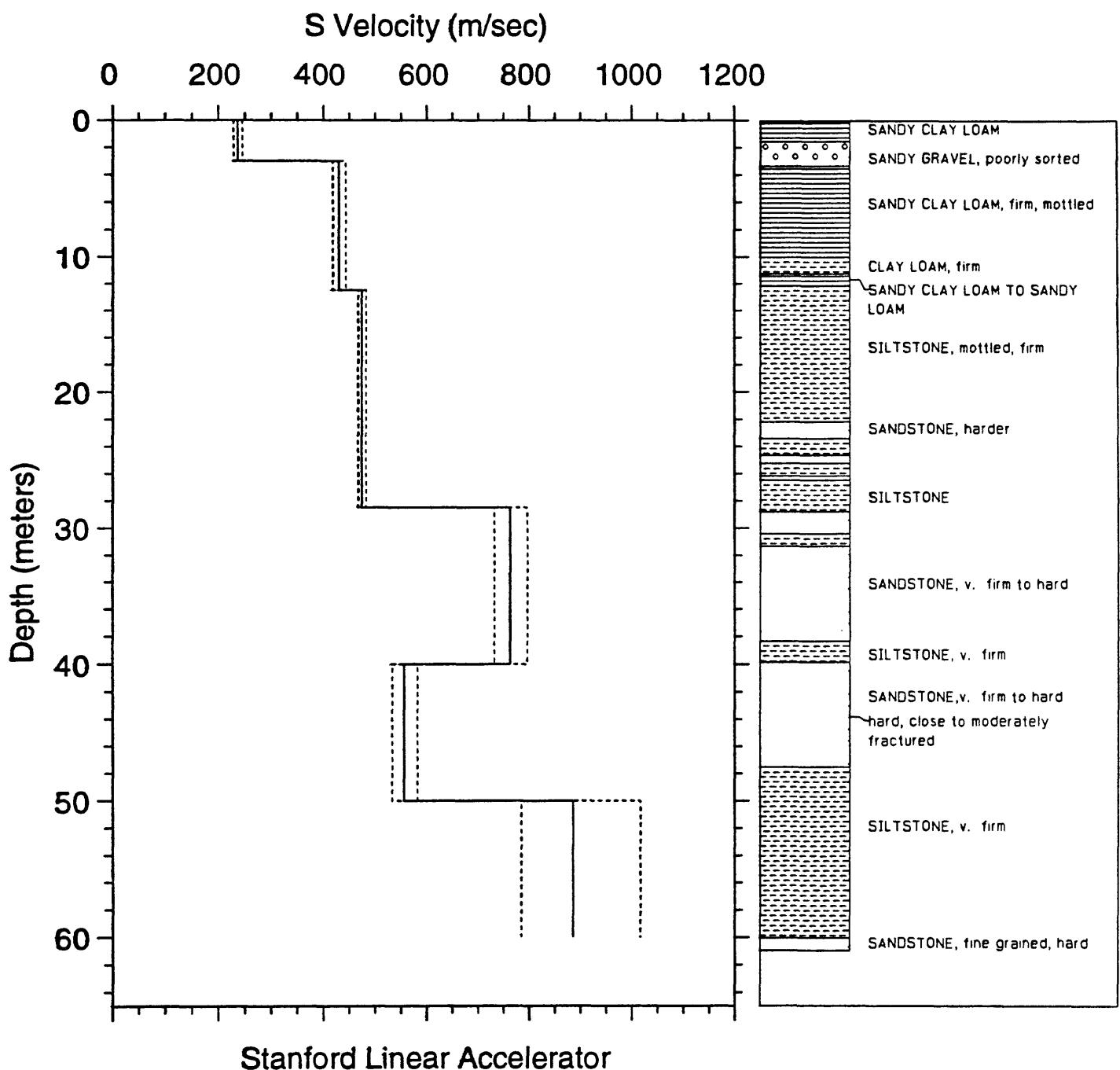


Figure 53. S-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

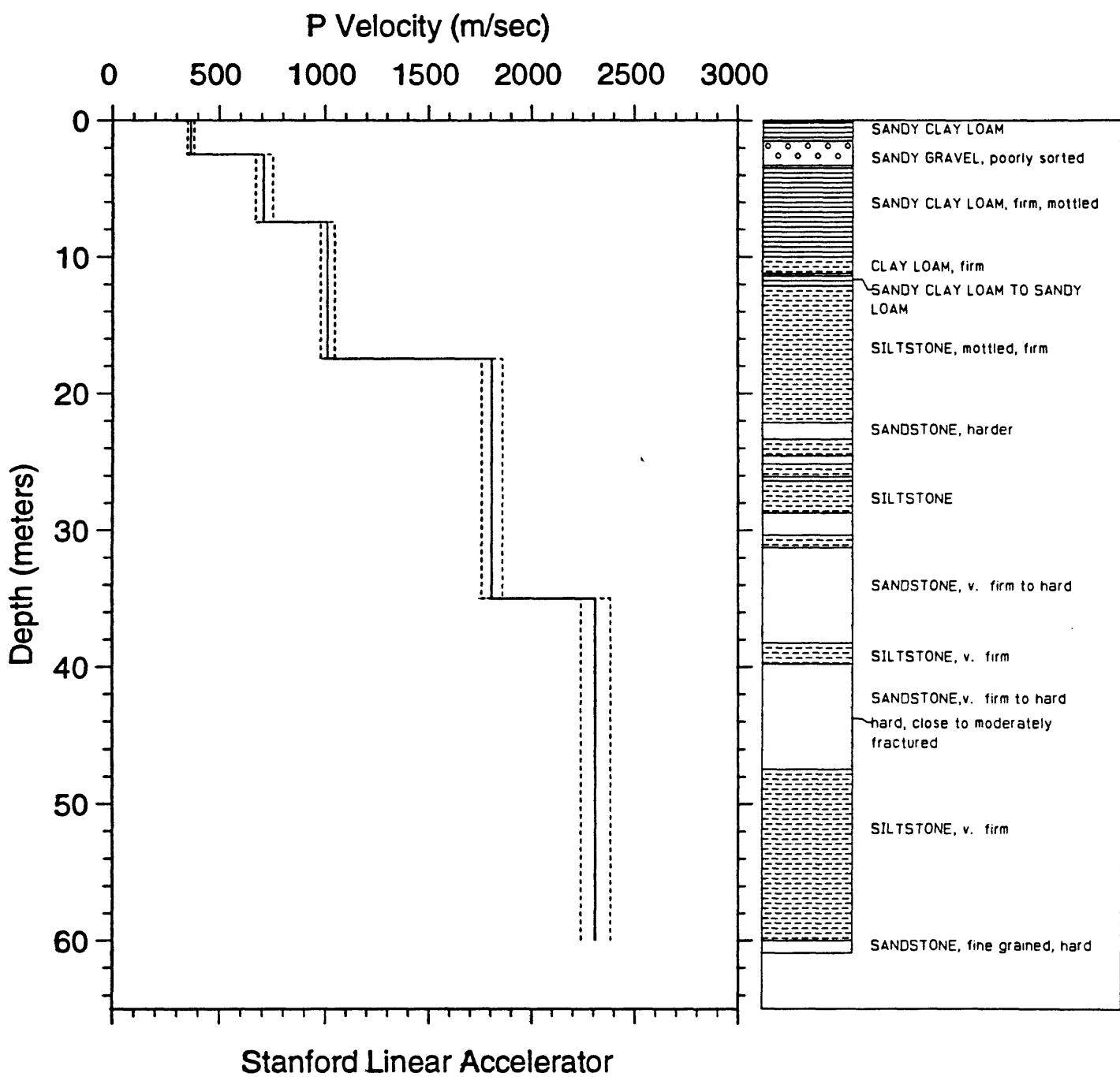


Figure 54. P-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

TABLE 11. S-wave arrival times and velocity summaries for SLAC site.

d(m)	d(ft)	t(sec)	sig	rsdl/sig	dtb(m)	dtb(ft)	ttb(s)	v(m/s)	vu(m/s)	vl(m/s)	vt(ft/s)	vu(ft/s)
2.5	8.2	.0104	1	-.2	0	0	.000	236	228	245	774	767
5.0	16.4	.0174	1	-.0	3.0	9.8	.013	236	228	245	774	803
7.5	24.6	.0237	1	-.5	12.5	41.0	.035	430	418	443	1411	1452
10.0	32.8	.0285	1	-.5	28.5	93.5	.069	474	467	482	1556	1532
12.5	41.0	.0343	1	-.5	40.0	131.2	.084	763	732	797	2504	1582
15.0	49.2	.0404	1	-.3	50.0	164.0	.102	556	533	582	1824	2402
17.5	57.4	.0462	1	-.8	60.0	196.9	.113	886	785	1017	2908	1908
20.0	65.6	.0509	1	-.3							2576	3337
22.5	73.8	.0556	1	-.3								
25.0	82.0	.0600	1	-.1.2								
27.5	90.2	.0663	1	-.1								
30.0	98.4	.0714	1	-.9								
32.5	106.6	.0740	1	-.2								
35.0	114.8	.0757	2	-.9								
37.5	123.0	.0808	1	-.5								
40.0	131.2	.0840	2	-.3								
42.5	139.4	.0870	2	-.7								
45.0	147.6	.0923	4	-.1								
47.5	155.8	.0970	1	-.1								
50.0	164.0	.1020	2	-.3								
52.5	172.2	.1047	3	-.1								
55.0	180.4	.1069	4	-.1								
57.5	188.6	.1099	3	-.1								
60.0	196.9	.1130	3	-.0								

Explanation:

d(m) = depth in meters

d(ft) = depth in feet

t(sec) = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)

sig = sigma, standard deviation normalized to the standard deviation of best picks

rsdl/sig = least-squares residual divided by sigma

dtb(m) = depth to bottom of layer in meters

dtb(ft) = depth to bottom of layer in feet

ttb(s) = arrival time in seconds to bottom of layer

v(m/s) = velocity in meters per second

· vl(m/s) = lower limit of velocity in meters per second *

vu(m/s) = upper limit of velocity in meters per second

vt(ft/s) = velocity in feet per second

vl(ft/s) = lower limit of velocity in feet per second

vu(ft/s) = upper limit of velocity in feet per second

* see text for explanation of velocity limits

TABLE 12. P-wave arrival times and velocity summaries for SLAC site.

d(m)	d(ft)	t(sec)	sig	rsdl	dtb(m)	dtb(ft)	ttb(s)	v(m/s)	vl(m/s)	vu(m/s)	vl(ft/s)	vu(ft/s)
2.5	8.2	.0068	1	.1	.0	.0	.000	365	350	381	1196	1147
5.0	16.4	.0105	1	.1	2.5	8.2	.007	365	350	381	1196	1147
7.5	24.6	.0138	1	.1	7.5	24.6	.014	708	670	751	2323	2197
10.0	32.8	.0162	1	.2	17.5	57.4	.024	1011	978	1045	3315	2464
12.5	41.0	.0191	1	.2	35.0	114.8	.034	1805	1756	1857	5921	3429
15.0	49.2	.0217	1	.4	60.0	196.9	.044	2308	2238	2382	7571	6092
17.5	57.4	.0243	1	.5							7342	7816
20.0	65.6	.0250	1	.2								
22.5	73.8	.0260	1	.6								
25.0	82.0	.0275	1	.5								
27.5	90.2	.0291	1	.3								
30.0	98.4	.0312	1	.5								
32.5	106.6	.0322	1	.1								
35.0	114.8	.0334	1	.1								
37.5	123.0	.0349	1	.3								
40.0	131.2	.0358	1	.1								
42.5	139.4	.0367	1	.1								
45.0	147.6	.0380	1	.2								
47.5	155.8	.0389	1	.0								
50.0	164.0	.0398	1	.2								
52.5	172.2	.0410	1	.1								
55.0	180.4	.0419	1	.3								
57.5	188.6	.0440	2	.4								
60.0	196.9	.0458	3	.5								

Explanation:

d(m) = depth in meters

d(ft) = depth in feet

t(sec) = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)

sig = sigma, standard deviation normalized to the standard deviation of best picks

rsdl/sig = least-squares residual divided by sigma

dtb(m) = depth to bottom of layer in meters

dtb(ft) = depth to bottom of layer in feet

ttb(s) = arrival time in seconds to bottom of layer

vl(m/s) = velocity in meters per second

vl(m/s) = lower limit of velocity in meters per second *

vu(m/s) = upper limit of velocity in meters per second

vl(ft/s) = velocity in feet per second

vl(ft/s) = lower limit of velocity in feet per second

vu(ft/s) = upper limit of velocity in feet per second

* see text for explanation of velocity limits

UNITED STATES
DEPARTMENT OF THE INTERIOR
GEOLOGICAL SURVEY

MOUNTAIN VIEW QUADRANGLE
CALIFORNIA
7.5 MINUTE SERIES (TOPOGRAPHIC)



SCALE 1:24 000

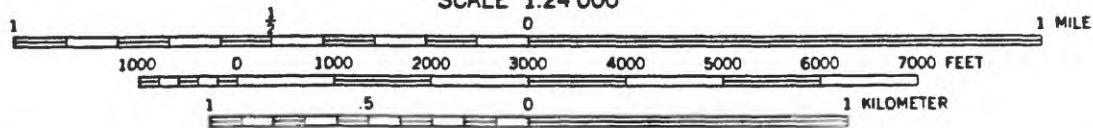


Figure 55. Site location map for Sunnyvale Colton Avenue borehole. The borehole is located approximately 100 meters from the strong-motion recorder.

Definitions of terms used for descriptions of sedimentary deposits and bedrock materials

Rock hardness: response to hand and geologic hammer: (Ellen et al., 1972)

hard - hammer bounces off with solid sound
 firm - hammer dents with thud, pick point dents or penetrates slightly
 soft - pick points penetrates
 friable material can be crumbled into individual grains by hand.

Fracture spacing: (Ellen et al., 1972)

cm	in	fracture spacing
0-1	0-1/2	v. close
1-5	1/2-2	close
5-30	2-12	moderate
30-100	12-36	wide
> 100	> 36	v. wide

Weathering:

Fresh: no visible signs of weathering

Slight: no visible decomposition of minerals, slight discoloration

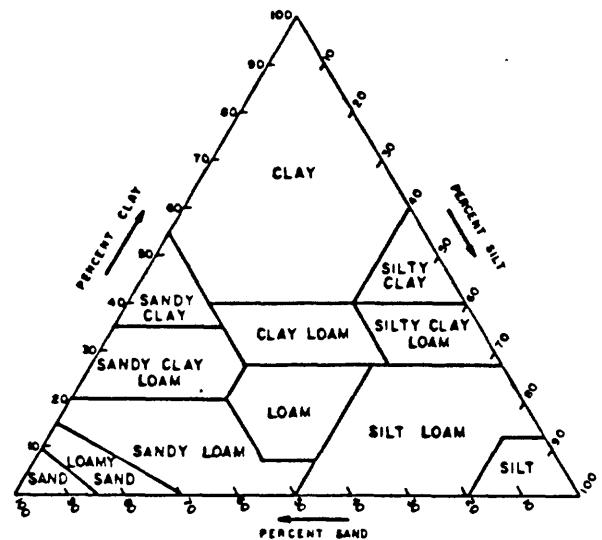
Moderate: slight decomposition of minerals and disintegration of rock, deep and thorough discoloration

Deep: extensive decomposition of minerals and complete disintegration of rock but original structure is preserved.

Relative density of sand and consistency of clay is correlated with penetration resistance: (Terzaghi and Peck, 1948)

blows/ft.	relative density	blows/ft.	consistency
0-4	v. loose	<2	v. soft
4-10	loose	2-4	soft
10-30	medium	4-8	medium
30-50	dense	8-15	stiff
>50	v. dense	15-30	v. stiff
		>30	hard

Texture: the relative proportions of clay, silt, and sand below 2mm. Proportions of larger particles are indicated by modifiers of textural class names. Determination is made in the field mainly by feeling the moist soil (Soil Survey, Staff, 1951).



Color: Standard Munsell color names are given for the dominant color of the moist soil and for prominent mottles.

Types of samples

SP - Standard Penetration 1 + 3/8 in in ID sampler)

S - Thin-wall push sampler

O - Osterberg fixed-piston sampler

P - Pitcher Barrel sampler

CH - California Penetration (2 in ID sampler)

DC - Diamond Core

Figure 56. Explanation of geologic log.

SITE: SUNNYVALE COLTON AVENUE

DATE: 1/28/91

BLows/ FOOT	SAMPLE TYPE	GRAPHIC LOG	DEPTH (ft) (m)	LATITUDE: 37 24.170 LONGITUDE: 122 01.488 DESCRIPTION
			0	SILTY CLAY, v. dk. grey (2.5Y 3/1)
			1	
			2	GRAVELLY SAND, dk. olive grey (5Y 3/2), moderately well sorted
			3	coarser
			4	SILTY CLAY LOAM, yellowish brown (10YR 5/4)
			5	SANDY CLAY LOAM, olive brown (2.5Y 4/4)
			6	CLAY LOAM, olive brown (2.5Y 4/4), stiffer
			7	SAND
			8	SILTY CLAY LOAM, dk greyish brown (2.5Y 4/2)
			9	sandy
			10	
			11	GRAVELLY SAND
			12	
			13	SANDY GRAVEL
			14	
			15	
			50	

Figure 57. Geologic log of the Sunnyvale Colton Avenue borehole.

SITE: SUNNYVALE COLTON AVENUE

DATE: 1/28/91

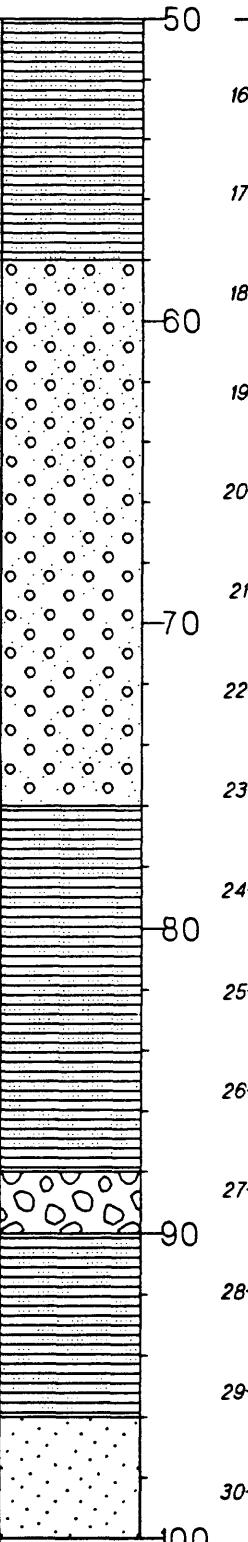
BLOWS/ FOOT	SAMPLE TYPE	GRAPHIC LOG	DEPTH (ft) (m)	DESCRIPTION
				LATITUDE: 37 24.170 LONGITUDE: 122 01.488
				DESCRIPTION
				
			50	CLAY LOAM TO SANDY CLAY LOAM, brown (10YR 5/3), stiffer
			16	
			17	
			18	GRAVELLY SAND
			19	
			20	
			21	
			22	
			23	CLAY LOAM, v. stiff
			24	
			25	
			26	
			27	GRAVEL
			28	SANDY CLAY LOAM
			29	sandier
			30	
			100	SAND, moderately well sorted, some fine gravel

Figure 57. (Continued).

SITE: SUNNYVALE COLTON AVENUE

DATE: 1/28/91

BLOWS/ FOOT	SAMPLE TYPE	GRAPHIC LOG	DEPTH (ft) (m)	DESCRIPTION
				LATITUDE: 37 24.170 LONGITUDE: 122 01.488
				DESCRIPTION
			100	
			31	SILTY CLAY TO CLAY LOAM, olive gray (5Y 4/2)
			32	SAND
			33	
			110	
			34	SILTY CLAY, olive gray, v. stiff
			35	GRAVELLY SAND
			36	SILTY CLAY
			120	
			37	SAND
			38	
			39	
			130	
			40	SILTY LOAM TO SILTY CLAY LOAM, dk greenish grey (5GY 4/1), stiff
			41	
			42	
			140	v. stiff
			43	
			44	
			45	
			150	

Figure 57. (Continued).

SITE: SUNNYVALE COLTON AVENUE

DATE: 1/28/91

Figure 57. (Continued).

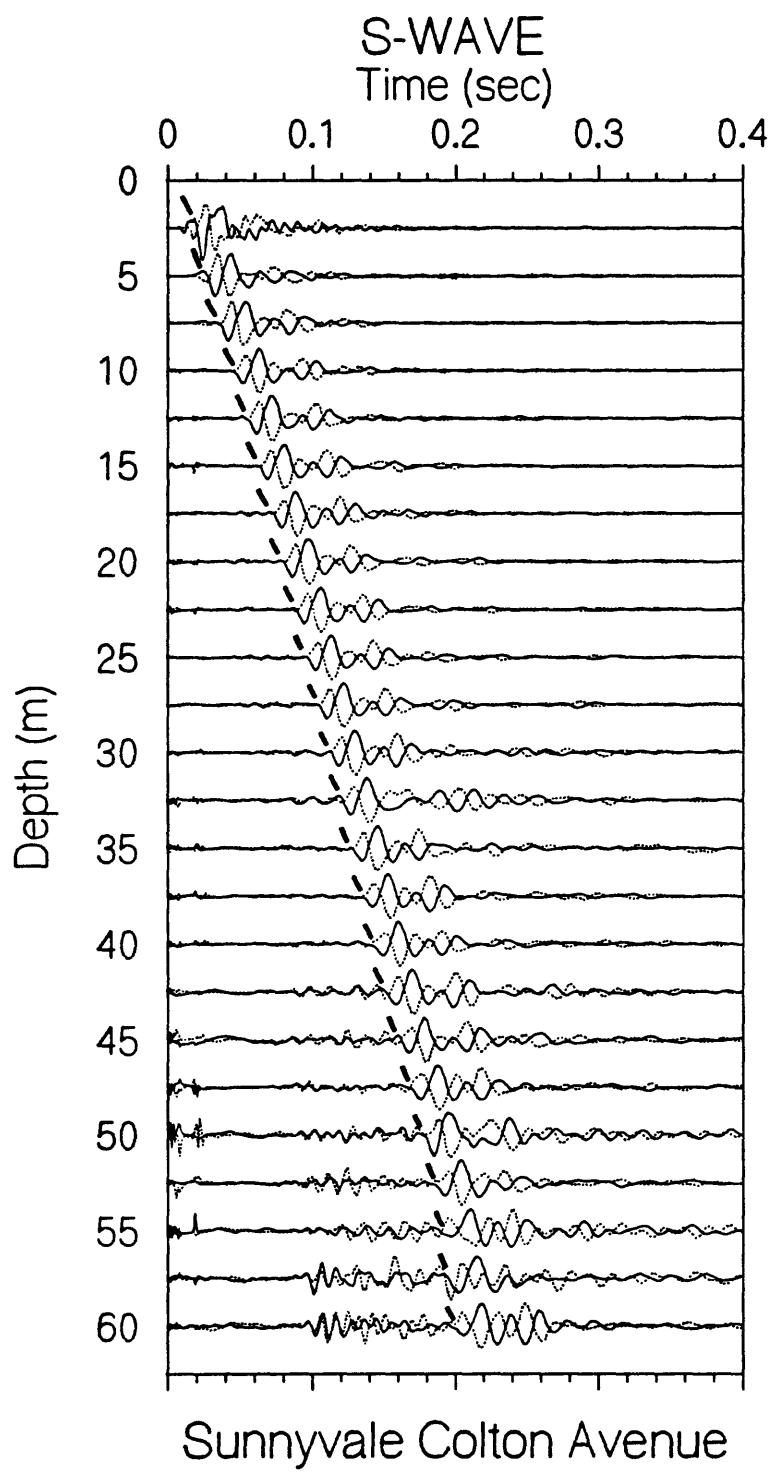


Figure 58. Horizontal-component record section (from horizontal impacts in opposite directions) superimposed for identification of S-wave onset. Approximate S-wave picks are indicated by the accent marks.

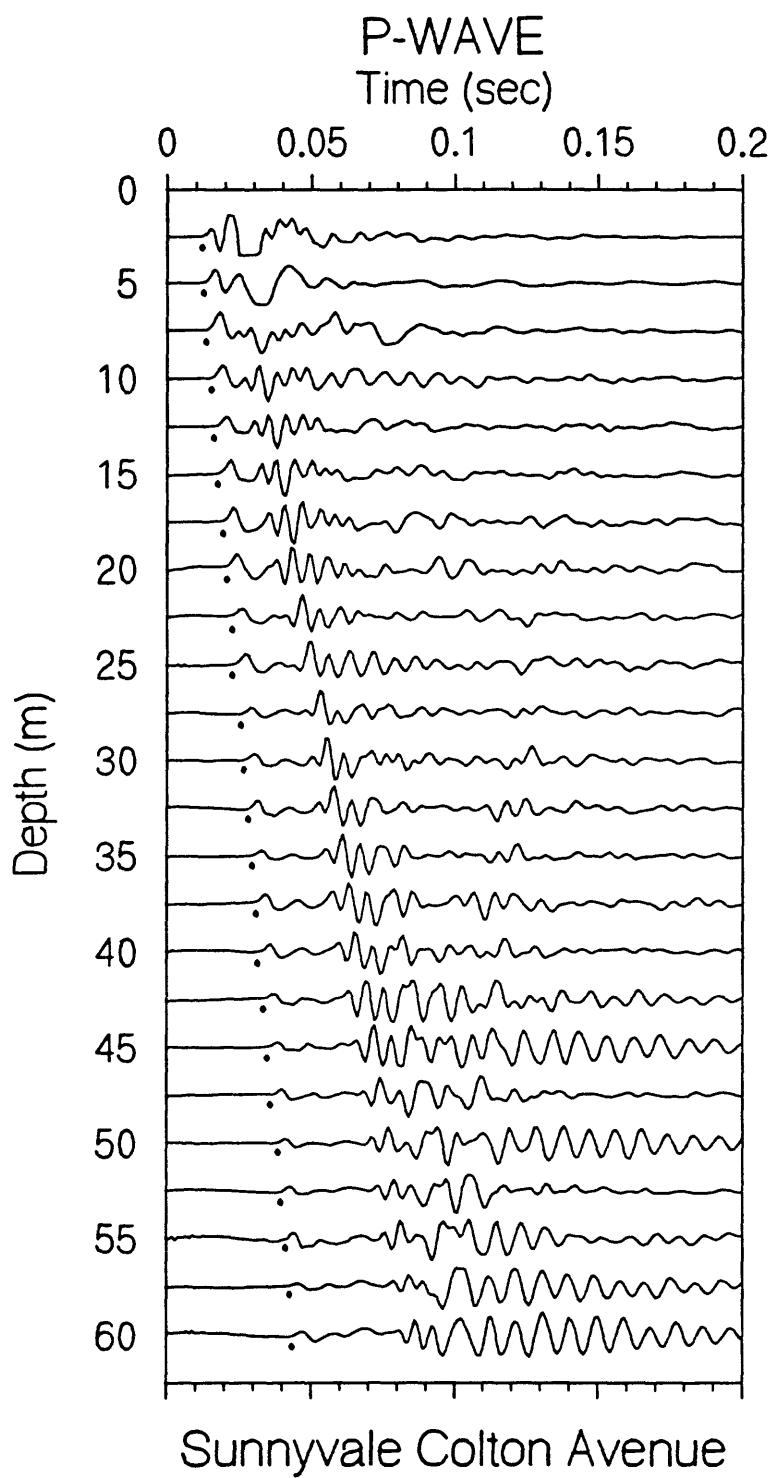


Figure 59. Vertical-component record section. P-wave arrivals are shown by the solid circles.

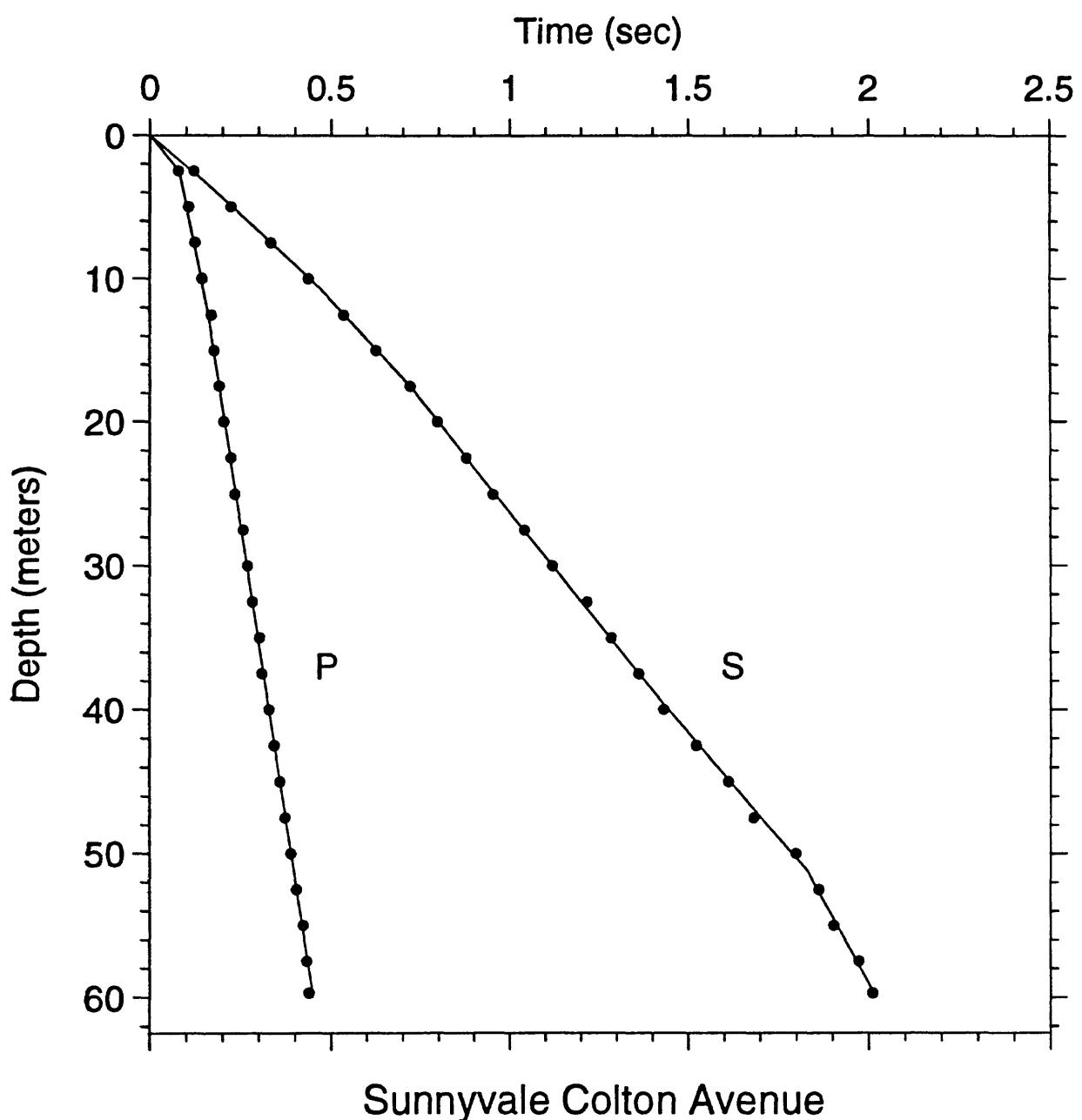


Figure 60. Time-depth graph of P-wave and S-wave picks. Line segments show the hinged-least-squares fit to the data points.

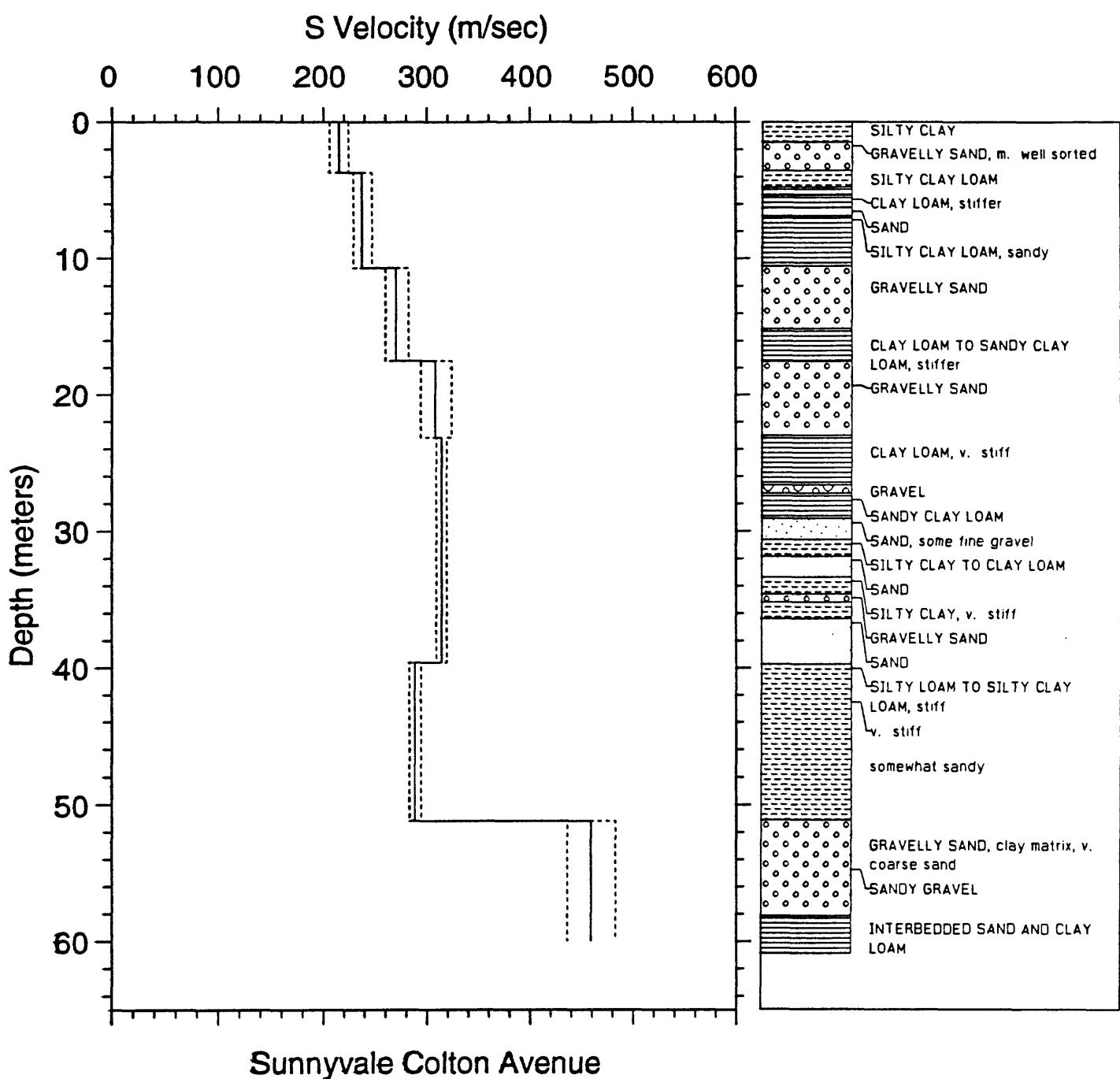


Figure 61. S-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

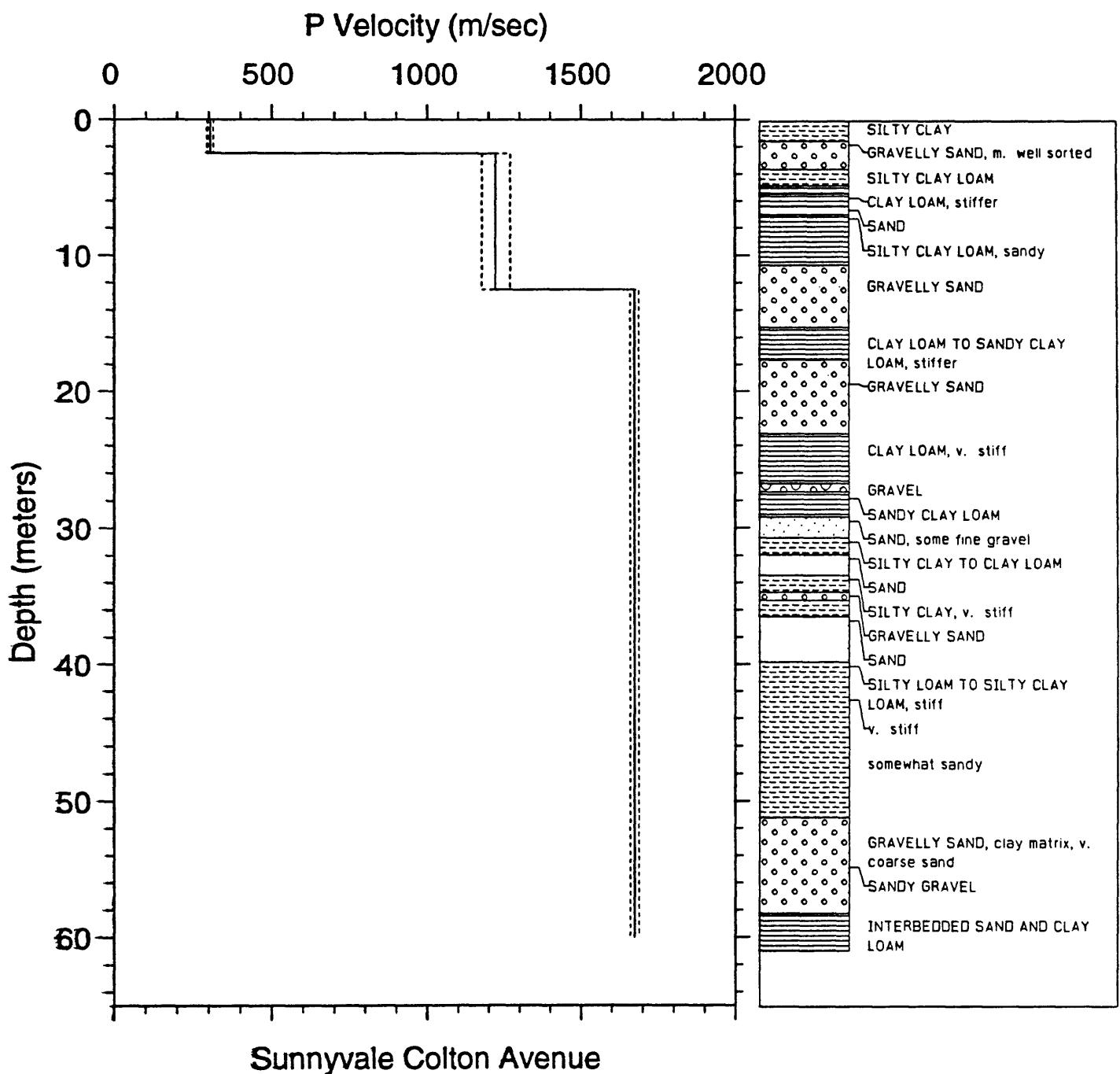


Figure 62. P-wave velocity profiles with dashed lines representing plus and minus one standard deviation. Simplified geologic log is shown for correlation with velocities.

TABLE 13. S-wave arrival times and velocity summaries for Sunnyvale Colton Avenue site.

d(m)	d(ft)	t(sec)	sig	rsdl/sig	dtb(m)	dtb(ft)	ttb(s)	v(m/s)	vl(m/s)	vu(m/s)	v(ft/s)	vl(ft/s)	vu(ft/s)
2.5	8.2	.0121	1	.5	.0	.0	.000	215	206	224	675	704	736
5.0	16.4	.0223	1	.4	3.7	12.1	.017	215	206	224	675	704	736
7.5	24.6	.0333	1	.4	10.7	35.1	.047	237	229	247	779	750	811
10.0	32.8	.0437	1	.1	17.5	57.4	.072	270	260	282	887	852	925
12.5	41.0	.0535	1	.1	23.2	76.1	.090	308	294	324	1011	965	1062
15.0	49.2	.0626	1	.0	39.6	129.9	.143	314	309	319	1030	1014	1046
17.5	57.4	.0720	1	.1	51.2	168.0	.183	288	283	294	946	927	965
20.0	65.6	.0797	1	.3	60.0	196.9	.202	459	436	483	1505	1432	1586
22.5	73.8	.0878	1	.3									
25.0	82.0	.0953	1	.8									
27.5	90.2	.1042	1	.2									
30.0	98.4	.1120	1	.0									
32.5	106.6	.1217	1	1.7									
35.0	114.8	.1285	1	.6									
37.5	123.0	.1362	1	.3									
40.0	131.2	.1432	1	.8									
42.5	139.4	.1522	1	.5									
45.0	147.6	.1612	1	.1									
47.5	155.8	.1683	1	1.7									
50.0	164.0	.1799	1	1.2									
52.5	172.2	.1863	1	.6									
55.0	180.4	.1903	1	.8									
57.5	188.6	.1973	1	.7									
59.7	195.9	.2011	1	.3									

Explanation:

rsdl/sig = depth in meters

d(ft) = depth in feet

t(sec) = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)

sig = sigma, standard deviation normalized to the standard deviation of best picks

rsdl/sig = least-squares residual divided by sigma

dtb(m) = depth to bottom of layer in meters

dtb(ft) = depth to bottom of layer in feet

ttb(s) = arrival time in seconds to bottom of layer

v(m/s) = velocity in meters per second

vl(m/s) = lower limit of velocity in meters per second *

vu(m/s) = upper limit of velocity in meters per second

v(ft/s) = velocity in feet per second

vl(ft/s) = lower limit of velocity in feet per second

vu(ft/s) = upper limit of velocity in feet per second

* see text for explanation of velocity limits

TABLE 14. P-wave arrival times and velocity summaries for Sunnyvale Colton Avenue site.

d(m)	d(ft)	t(sec)	sig	rsdl/sig	dtb(m)	dtb(ft)	ttb(s)	v(m/s)	vl(m/s)	vu(m/s)	v(ft/s)	vl(ft/s)	vu(ft/s)
2.5	8.2	.0079	1	-.3	.0	.0	.000	304	295	314	999	969	1030
5.0	16.4	.0106	1	-.3	2.5	8.2	.008	304	295	314	999	969	1030
7.5	26.6	.0124	1	-.1	12.5	41.0	.016	1222	1177	1270	4009	3862	4167
10.0	32.8	.0144	1	-.0	60.0	196.9	.045	1674	1660	1688	5491	5445	5538
12.5	41.0	.0169	1	-.5									
15.0	49.2	.0177	1	-.2									
17.5	57.4	.0191	1	-.3									
20.0	65.6	.0204	1	-.5									
22.5	73.8	.0224	1	-.0									
25.0	82.0	.0234	1	-.5									
27.5	90.2	.0258	1	-.4									
30.0	98.4	.0269	1	-.0									
32.5	106.6	.0283	1	-.0									
35.0	114.8	.0303	1	-.5									
37.5	123.0	.0309	1	-.4									
40.0	131.2	.0329	1	-.1									
42.5	139.4	.0343	1	-.0									
45.0	147.6	.0359	1	-.1									
47.5	155.8	.0373	1	-.0									
50.0	164.0	.0389	1	-.1									
52.5	172.2	.0405	1	-.2									
55.0	180.4	.0423	1	.5									
57.5	188.6	.0433	1	-.0									
59.7	195.9	.0439	1	-.7									

Explanation:

d(m) = depth in meters

d(ft) = depth in feet

t(sec) = arrival time in seconds (S-wave arrival times are the average of picks from traces obtained from hammer blows differing in direction by 180°)

sig = sigma, standard deviation normalized to the standard deviation of best picks

rsdl/sig = least-squares residual divided by sigma

dtb(m) = depth to bottom of layer in meters

dtb(ft) = depth to bottom of layer in feet

ttb(s) = arrival time in seconds to bottom of layer

v(m/s) = velocity in meters per second

vl(m/s) = lower limit of velocity in meters per second *

vu(m/s) = upper limit of velocity in meters per second

v(ft/s) = velocity in feet per second

vl(ft/s) = lower limit of velocity in feet per second

vu(ft/s) = upper limit of velocity in feet per second

* see text for explanation of velocity limits